



CALCULATION PACKAGE

November 7, 2022

JayMarc Homes Spring Residence

Mercer Island, Washington

MULHERN & KULP STRUCTURAL ENGINEERING, INC.

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Signature, Seal & Date



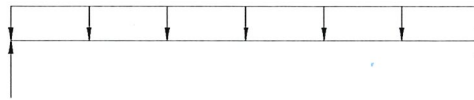
BEAM & HEADER CALCULATIONS

BEAM DESCRIPTION: RF FRMG - TYP EXT HDR - WORST CASE LENGTH

B1

PARAMETERS:

L = FT
W = KLF
P = K



ANALYSIS:

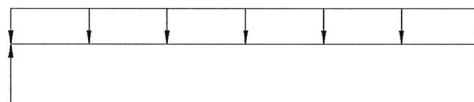
$R_{MAX} =$ K $V_D =$ K < $V_{ALL} =$ K ADEQUATE
 $M_{MAX} =$ K-FT < $M_{ALL} =$ K-FT ($C_D=1.15$) ADEQUATE
 $\Delta_{TL} =$ IN. $L/$ < $L/24D$ ADEQUATE

BEAM DESCRIPTION: RF FRMG - TYP EXT HDR - WORST CASE LOAD

B1

PARAMETERS:

L = FT
W = KLF
P = K



ANALYSIS:

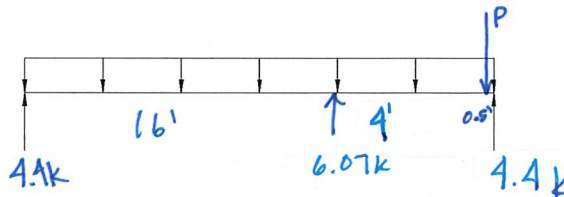
$R_{MAX} =$ K $V_D =$ K < $V_{ALL} =$ K ADEQUATE
 $M_{MAX} =$ K-FT < $M_{ALL} =$ K-FT ($C_D=1.15$) ADEQUATE
 $\Delta_{TL} =$ IN. $L/$ < $L/24D$ ADEQUATE

BEAM DESCRIPTION: RF FRMG - FLUSH BTM BM 2-SPAN @ BED 2

B2

PARAMETERS:

L = FT
W = KLF
P = K



ANALYSIS:

$R_{MAX} =$ K $V_D =$ K < $V_{ALL} =$ K ADEQUATE
 $M_{MAX} =$ K-FT < $M_{ALL} =$ K-FT ($C_D=1.15$) ADEQUATE
 $\Delta_{TL} =$ IN. $L/$ < $L/24D$ ADEQUATE



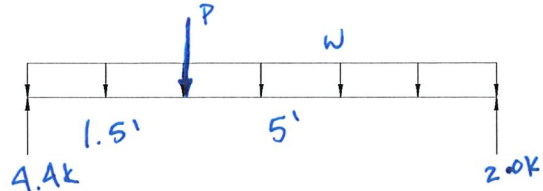
BEAM & HEADER CALCULATIONS

BEAM DESCRIPTION: RF FRMG - INT HDR @ BONUS

B3

PARAMETERS:

L = FT
W = KLF
P = K



ANALYSIS:

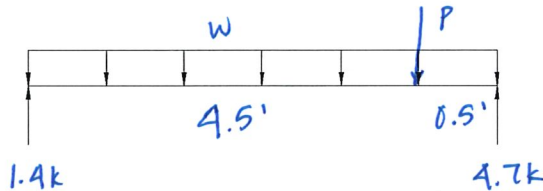
$R_{MAX} =$ K $V_D =$ K $< V_{ALL} =$ K ADEQUATE
 $M_{MAX} =$ K-FT $< M_{ALL} =$ K-FT ($C_D=1.15$) ADEQUATE
 $\Delta_{TL} =$ IN. $L/$ $< L/240$ ADEQUATE

BEAM DESCRIPTION: RF FRMG - FLUSH BTM BM @ HALL

B4

PARAMETERS:

L = FT
W = KLF
P = K



ANALYSIS:

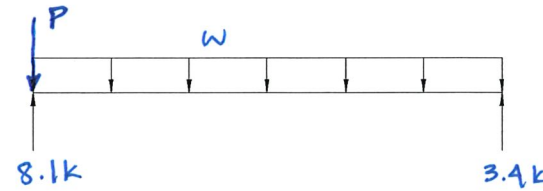
$R_{MAX} =$ K $V_D =$ K $< V_{ALL} =$ K ADEQUATE
 $M_{MAX} =$ K-FT $< M_{ALL} =$ K-FT ($C_D=1.15$) ADEQUATE
 $\Delta_{TL} =$ IN. $L/$ $< L/240$ ADEQUATE

BEAM DESCRIPTION: RF FRMG - DROPPED BM @ BED 3/4 ENTRY

B5

PARAMETERS:

L = FT
W = KLF
P = K



ANALYSIS:

$R_{MAX} =$ K $V_D =$ K $< V_{ALL} =$ K ADEQUATE
 $M_{MAX} =$ K-FT $< M_{ALL} =$ K-FT ($C_D=1.15$) ADEQUATE
 $\Delta_{TL} =$ IN. $L/$ $< L/240$ ADEQUATE



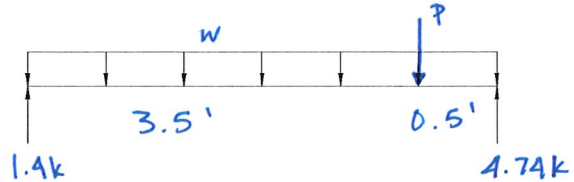
BEAM & HEADER CALCULATIONS

BEAM DESCRIPTION: RF FRMG - FLUSH BTTM BM @ PRIMARY BED

B6

PARAMETERS:

L = FT
W = KLF
P = K



ANALYSIS:

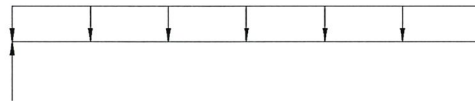
$R_{MAX} =$ K $V_D =$ K $< V_{ALL} =$ K ADEQUATE
 $M_{MAX} =$ K-FT $< M_{ALL} =$ K-FT ($C_D=1.15$) ADEQUATE
 $\Delta_{TL} =$ IN. $L/$ $< L/240$ ADEQUATE

BEAM DESCRIPTION: 2ND FLR FRMG - DROPPED BM @ REAR COVERED PATIO

B7

PARAMETERS:

L = FT
W = KLF
P = K



ANALYSIS:

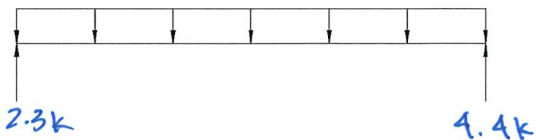
$R_{MAX} =$ K $V_D =$ K $< V_{ALL} =$ K ADEQUATE
 $M_{MAX} =$ K-FT $< M_{ALL} =$ K-FT ($C_D=1.15$) ADEQUATE
 $\Delta_{TL} =$ IN. $L/$ $< L/240$ ADEQUATE

BEAM DESCRIPTION: 2ND FLR FRMG - DROPPED BM @ SIDE COVERED PATIO REAR

B8

PARAMETERS:

L = FT
W = KLF
P = K



ANALYSIS:

$R_{MAX} =$ K $V_D =$ K $< V_{ALL} =$ K ADEQUATE
 $M_{MAX} =$ K-FT $< M_{ALL} =$ K-FT ($C_D=1.15$) ADEQUATE
 $\Delta_{TL} =$ IN. $L/$ $< L/240$ ADEQUATE



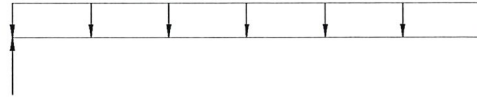
BEAM & HEADER CALCULATIONS

BEAM DESCRIPTION: 2ND FLR FRMG - DROPPED BM @ MIDDLE COVERED PATIO

B9

PARAMETERS:

L = FT
W = KLF
P = K



ANALYSIS:

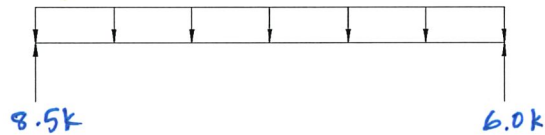
$R_{MAX} =$ K $V_D =$ K $< V_{ALL} =$ K ADEQUATE
 $M_{MAX} =$ K-FT $< M_{ALL} =$ K-FT ($C_D=1.0$) ADEQUATE
 $\Delta_{TL} =$ IN. $L/$ $< L/240$ ADEQUATE

BEAM DESCRIPTION: 2ND FLR FRMG - DROPPED BM @ SIDE COVERED PATIO FRONT

B10

PARAMETERS:

L = FT
W = KLF
P = K



ANALYSIS:

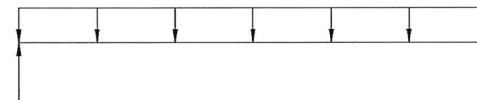
$R_{MAX} =$ K $V_D =$ K $< V_{ALL} =$ K ADEQUATE
 $M_{MAX} =$ K-FT $< M_{ALL} =$ K-FT ($C_D=$) ADEQUATE
 $\Delta_{TL} =$ IN. $L/$ $< L/240$ ADEQUATE

BEAM DESCRIPTION: 2ND FLR FRMG - DROPPED BM @ FRONT COVERED PATIO

B11

PARAMETERS:

L = FT
W = KLF
P = K



ANALYSIS:

$R_{MAX} =$ K $V_D =$ K $< V_{ALL} =$ K ADEQUATE
 $M_{MAX} =$ K-FT $< M_{ALL} =$ K-FT ($C_D=1.0$) ADEQUATE
 $\Delta_{TL} =$ IN. $L/$ $< L/240$ ADEQUATE



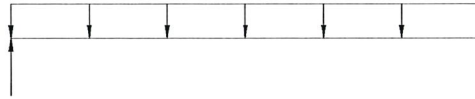
BEAM & HEADER CALCULATIONS

BEAM DESCRIPTION: 2ND FLR FRMG - FLUSH BM @ FRONT PORCH

B12

PARAMETERS:

L = FT
W = KLF
P = K



ANALYSIS:

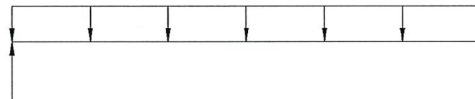
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 $M_{MAX} =$ K-FT $< M_{ALL} =$ K-FT ($C_D=1.15$) ADEQUATE
 $\Delta_{TL} =$ IN. $L/$ $< L/240$ ADEQUATE

BEAM DESCRIPTION: 2ND FLR FRMG - FLUSH BTM BM @ STAIRS

B13

PARAMETERS:

L = FT
W = KLF
P = K



ANALYSIS:

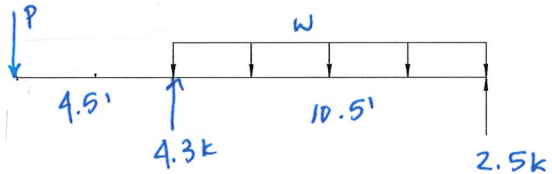
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 $M_{MAX} =$ K-FT $< M_{ALL} =$ K-FT ($C_D=1.0$) ADEQUATE
 $\Delta_{TL} =$ IN. $L/$ $< L/240$ ADEQUATE

BEAM DESCRIPTION: 2ND FLR FRMG - FLUSH BTM BM CANT'D @ PORCH

B14

PARAMETERS:

L = FT
W = KLF
P = K



ANALYSIS:

$R_{MAX} =$ K $V_D =$ K $< V_{ALL} =$ K ADEQUATE
 $M_{MAX} =$ K-FT $< M_{ALL} =$ K-FT ($C_D=1.15$) ADEQUATE
 $\Delta_{TL} =$ IN. $2 L/$ $< L/240$ ADEQUATE



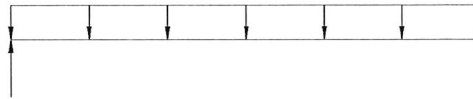
BEAM & HEADER CALCULATIONS

BEAM DESCRIPTION: 2ND FLR FRMG - DROPPED BM @ ENTRY / FOYER

B15

PARAMETERS:

L = FT
W = KLF
P = K



ANALYSIS:

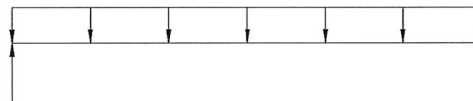
$R_{MAX} =$ K $V_D =$ K $< V_{ALL} =$ K ADEQUATE
 $M_{MAX} =$ K-FT $< M_{ALL} =$ K-FT ($C_D=1.0$) ADEQUATE
 $\Delta_{TL} =$ IN. $L/$ $< L/240$ ADEQUATE

BEAM DESCRIPTION: 2ND FLR FRMG - DROPPED BM @ FOYER / GREAT ROOM

B16

PARAMETERS:

L = FT
W = KLF
P = K



ANALYSIS:

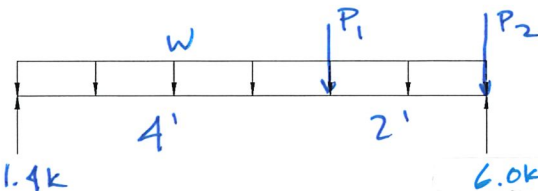
$R_{MAX} =$ K $V_D =$ K $< V_{ALL} =$ K ADEQUATE
 $M_{MAX} =$ K-FT $< M_{ALL} =$ K-FT ($C_D=1.0$) ADEQUATE
 $\Delta_{TL} =$ IN. $L/$ $< L/240$ ADEQUATE

BEAM DESCRIPTION: 2ND FLR FRMG - INT HDR @ WIC / FOYER

B17

PARAMETERS:

L = FT
W = KLF
 $P_1 =$ K $P_2 = 3.5$



ANALYSIS:

$R_{MAX} =$ K $V_D =$ K $< V_{ALL} =$ K ADEQUATE
 $M_{MAX} =$ K-FT $< M_{ALL} =$ K-FT ($C_D=1.15$) ADEQUATE
 $\Delta_{TL} =$ IN. $L/$ $< L/240$ ADEQUATE



BEAM & HEADER CALCULATIONS

BEAM DESCRIPTION: 2ND FLR FRMG - INT HDR @ PWDR / FOYER

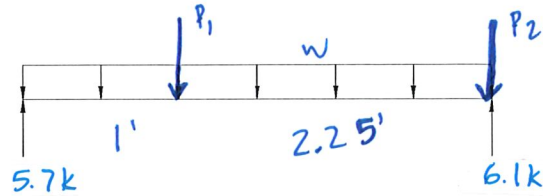
B18

PARAMETERS:

L = 3.25 FT

W = 0.073 KLF

P_F = 8.1 K P₂ = 3.5



ANALYSIS:

R_{MAX} = 6.1 K V_D = 5.7 K < V_{ALL} = 6.4 K

M_{MAX} = 5.67 K-FT < M_{ALL} = 10.9 K-FT (C_D=1.15)

Δ_{TL} = 0.022 IN. L/ 999+ < L/240

- ADEQUATE
- ADEQUATE
- ADEQUATE

3 1/2 x 9 GLB

BEAM DESCRIPTION: 2ND FLR FRMG - EXT SGD HDR @ GREAT ROOM

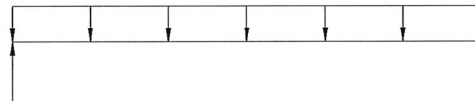
B19

PARAMETERS:

L = 15 FT

W = 0.073 KLF

P = -- K



ANALYSIS:

R_{MAX} = 0.55 K V_D = [] K < V_{ALL} = 7.2 K

M_{MAX} = 2.05 K-FT < M_{ALL} = 8.83 K-FT (C_D=1.0)

Δ_{TL} = 0.125 IN. L/ 999+ < L/240

- ADEQUATE
- ADEQUATE
- ADEQUATE

6 x 12

BEAM DESCRIPTION: 2ND FLR FRMG - EXT WNDW HDR @ STUDY

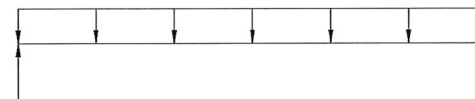
B20

PARAMETERS:

L = 10 FT

W = 0.67 KLF

P = -- K



ANALYSIS:

R_{MAX} = 3.4 K V_D = [] K < V_{ALL} = 8.24 K

M_{MAX} = 8.4 K-FT < M_{ALL} = 10.16 K-FT (C_D=1.15)

Δ_{TL} = 0.167 IN. L/ 719 < L/240

- ADEQUATE
- ADEQUATE
- ADEQUATE

6 x 12



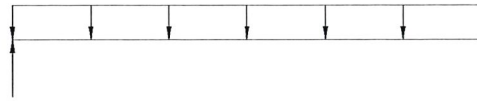
BEAM & HEADER CALCULATIONS

BEAM DESCRIPTION: 2ND FLR FRMG - INT HDR @ PANTRY & MUD

B21

PARAMETERS:

L = FT
W = KLF
P = K



ANALYSIS:

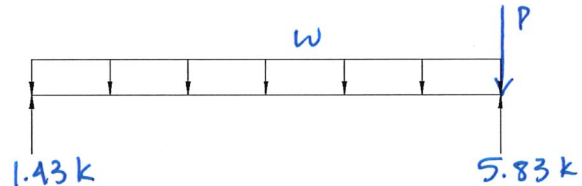
$R_{MAX} =$ K $V_D =$ K < $V_{ALL} =$ K ADEQUATE
 $M_{MAX} =$ K-FT < $M_{ALL} =$ K-FT ($C_D=1.0$) ADEQUATE
 $\Delta_{TL} =$ IN. $L/$ < $L/240$ ADEQUATE

BEAM DESCRIPTION: 2ND FLR FRMG - FLUSH BM @ MUD

B22

PARAMETERS:

L = FT
W = KLF
P = K



ANALYSIS:

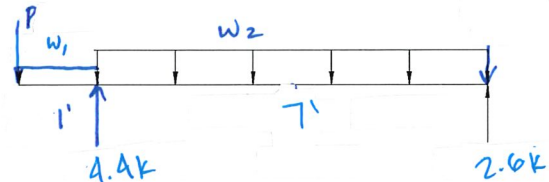
$R_{MAX} =$ K $V_D =$ K < $V_{ALL} =$ K ADEQUATE
 $M_{MAX} =$ K-FT < $M_{ALL} =$ K-FT ($C_D=1.15$) ADEQUATE
 $\Delta_{TL} =$ IN. $L/$ < $L/240$ ADEQUATE

BEAM DESCRIPTION: 2ND FLR FRMG - FLUSH BM CANT'D @ MUD / GARAGE

B23

PARAMETERS:

L = FT
 $W_1 =$ KLF $W_2=0.725$
P = K



ANALYSIS:

$R_{MAX} =$ K $V_D =$ K < $V_{ALL} =$ K ADEQUATE
 $M_{MAX} =$ K-FT < $M_{ALL} =$ K-FT ($C_D=1.15$) ADEQUATE
 $\Delta_{TL} =$ IN. $2L/$ < $L/240$ ADEQUATE



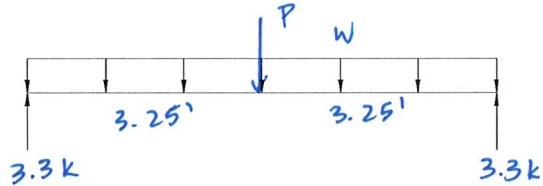
BEAM & HEADER CALCULATIONS

BEAM DESCRIPTION: 2ND FLR FRMG - FLUSH BM @ MUD / MECH

B24

PARAMETERS:

L = FT
W = KLF
P = K



ANALYSIS:

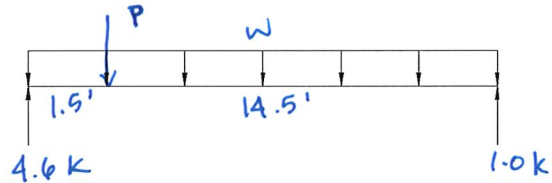
$R_{MAX} =$ K $V_D =$ K < $V_{ALL} =$ K ADEQUATE
 $M_{MAX} =$ K-FT < $M_{ALL} =$ K-FT ($C_D=1.15$) ADEQUATE
 $\Delta_{TL} =$ IN. $L/$ < $L/240$ ADEQUATE

BEAM DESCRIPTION: 2ND FLR FRMG - FLUSH BM @ KITCHEN

B25

PARAMETERS:

L = FT
W = KLF
P = K



ANALYSIS:

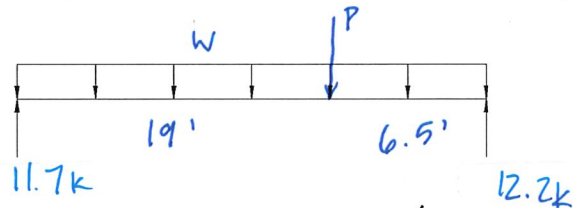
$R_{MAX} =$ K $V_D =$ K < $V_{ALL} =$ K ADEQUATE
 $M_{MAX} =$ K-FT < $M_{ALL} =$ K-FT ($C_D=1.15$) ADEQUATE
 $\Delta_{TL} =$ IN. $L/$ < $L/240$ ADEQUATE

BEAM DESCRIPTION: 2ND FLR FRMG - FLUSH BTM BM @ DINING / KITCHEN

B26

PARAMETERS:

L = FT
W = KLF
P = K



ANALYSIS:

$R_{MAX} =$ K $V_D =$ K < $V_{ALL} =$ K ADEQUATE
 $M_{MAX} =$ K-FT < $M_{ALL} =$ K-FT ($C_D=1.15$) ADEQUATE
 $\Delta_{TL} =$ IN. $L/$ < $L/240$ ADEQUATE



BEAM & HEADER CALCULATIONS

BEAM DESCRIPTION: 2ND FLR FRMG - FLUSH BM UNDER EXT WALL @ KITCHEN

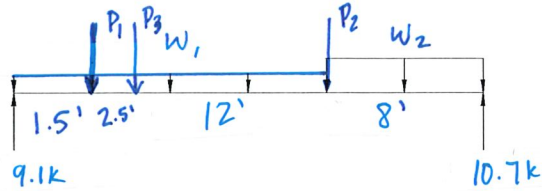
B27

PARAMETERS:

L = FT

W₁ = KLF W₂ = 0.23

P₁ = K P₂ = 12.2
P₃ = 2.15



ANALYSIS:

R_{MAX} = K V_D = K < V_{ALL} = K ADEQUATE

M_{MAX} = K-FT < M_{ALL} = K-FT (C_D=1.15) ADEQUATE

Δ_{TL} = IN. L/ < L/240 ADEQUATE

BEAM DESCRIPTION: 2ND FLR FRMG - EXT WNDW HDR @ DINING

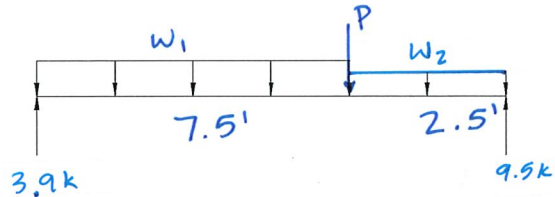
B28

PARAMETERS:

L = FT

W₁ = KLF W₂ = 0.084

P = K



ANALYSIS:

R_{MAX} = K V_D = K < V_{ALL} = K ADEQUATE

M_{MAX} = K-FT < M_{ALL} = K-FT (C_D=1.15) ADEQUATE

Δ_{TL} = IN. L/ < L/240 ADEQUATE

BEAM DESCRIPTION: 2ND FLR FRMG - FLUSH BM @ GARAGE

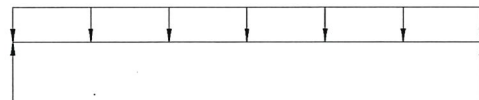
B29

PARAMETERS:

L = FT

W = KLF

P = K



ANALYSIS:

R_{MAX} = K V_D = K < V_{ALL} = K ADEQUATE

M_{MAX} = K-FT < M_{ALL} = K-FT (C_D=1.15) ADEQUATE

Δ_{TL} = IN. L/ < L/240 ADEQUATE



BEAM & HEADER CALCULATIONS

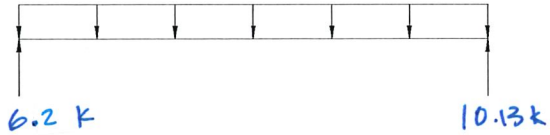
BEAM DESCRIPTION: 2ND FLR FRMG - FLUSH BTM BM @ GARAGE

B30

PARAMETERS:

L = FT
W = KLF
P = K

SEE
ENERCALC
OUTPUT



ANALYSIS:

$R_{MAX} =$ K $V_D =$ K < $V_{ALL} =$ K ADEQUATE
 $M_{MAX} =$ K-FT < $M_{ALL} =$ K-FT ($C_D=1.15$) ADEQUATE
 $\Delta_{TL} =$ IN. $L/$ < $L/240$ ADEQUATE

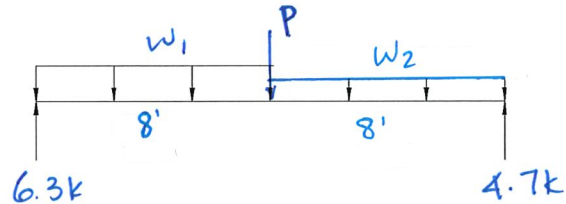
5'1/2 x 19'1/2 GLB

BEAM DESCRIPTION: 2ND FLR FRMG - EXT LONG GARAGE HDR

B31

PARAMETERS:

L = FT
 $W_1 =$ KLF $W_2 = 0.1$
P = K



ANALYSIS:

$R_{MAX} =$ K $V_D =$ K < $V_{ALL} =$ K ADEQUATE
 $M_{MAX} =$ K-FT < $M_{ALL} =$ K-FT ($C_D=1.15$) ADEQUATE
 $\Delta_{TL} =$ IN. $L/$ < $L/240$ ADEQUATE

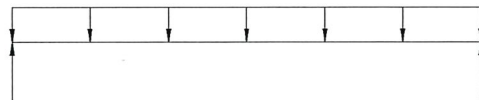
5'1/2 x 16'1/2 GLB

BEAM DESCRIPTION: 2ND FLR FRMG - EXT SHORT GARAGE HDR

B32

PARAMETERS:

L = FT
W = KLF
P = K



ANALYSIS:

$R_{MAX} =$ K $V_D =$ K < $V_{ALL} =$ K ADEQUATE
 $M_{MAX} =$ K-FT < $M_{ALL} =$ K-FT ($C_D=1.15$) ADEQUATE
 $\Delta_{TL} =$ IN. $L/$ < $L/240$ ADEQUATE

4 x 12



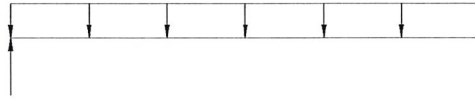
BEAM & HEADER CALCULATIONS

BEAM DESCRIPTION: 2ND FLR FRMG - TYP EXT HDR - WORST CASE LOAD

B33

PARAMETERS:

L = FT
W = KLF
P = K



ANALYSIS:

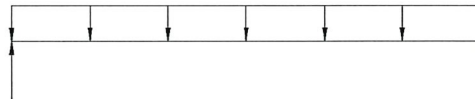
$R_{MAX} =$ K $V_D =$ K $< V_{ALL} =$ K ADEQUATE
 $M_{MAX} =$ K-FT $< M_{ALL} =$ K-FT ($C_D=1.0$) ADEQUATE
 $\Delta_{TL} =$ IN. $L/$ $< L/240$ ADEQUATE

BEAM DESCRIPTION: 2ND FLR FRMG - TYP EXT HDR - WORST CASE LENGTH

B33

PARAMETERS:

L = FT
W = KLF
P = K



ANALYSIS:

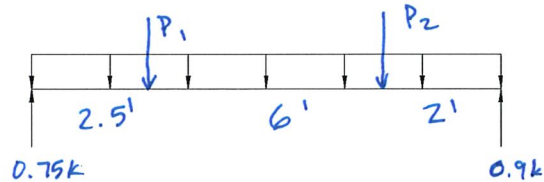
$R_{MAX} =$ K $V_D =$ K $< V_{ALL} =$ K ADEQUATE
 $M_{MAX} =$ K-FT $< M_{ALL} =$ K-FT ($C_D=1.0$) ADEQUATE
 $\Delta_{TL} =$ IN. $L/$ $< L/240$ ADEQUATE

BEAM DESCRIPTION: 2ND FLR FRMG - STAIR LANDING (LOW)

B34

PARAMETERS:

L = FT
W = KLF
 $P_1 =$ K $P_2 = 0.4$



ANALYSIS:

$R_{MAX} =$ K $V_D =$ K $< V_{ALL} =$ K ADEQUATE
 $M_{MAX} =$ K-FT $< M_{ALL} =$ K-FT ($C_D=1.0$) ADEQUATE
 $\Delta_{TL} =$ IN. $L/$ $< L/240$ ADEQUATE



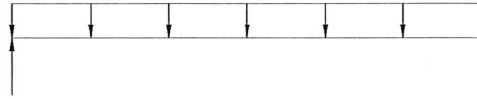
BEAM & HEADER CALCULATIONS

BEAM DESCRIPTION: 1ST FLR FRMG - TYP DROPPED CRAWL GRDR NO BRG

B35

PARAMETERS:

L = FT
W = KLF
P = K



ANALYSIS:

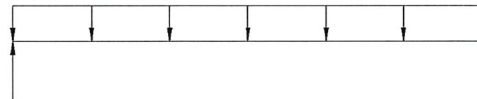
$R_{MAX} =$ K $V_D =$ K $< V_{ALL} =$ K ADEQUATE
 $M_{MAX} =$ K-FT $< M_{ALL} =$ K-FT ($C_D=1.0$) ADEQUATE
 $\Delta_{TL} =$ IN. $L/$ $< L/240$ ADEQUATE

BEAM DESCRIPTION: 1ST FLR FRMG - TYP DROPPED CRAWL GRDR BRG 1 LEVEL

B35

PARAMETERS:

L = FT
W = KLF
P = K



ANALYSIS:

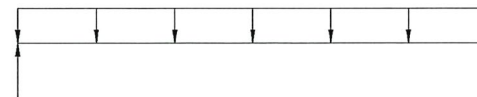
$R_{MAX} =$ K $V_D =$ K $< V_{ALL} =$ K ADEQUATE
 $M_{MAX} =$ K-FT $< M_{ALL} =$ K-FT ($C_D=1.0$) ADEQUATE
 $\Delta_{TL} =$ IN. $L/$ $< L/240$ ADEQUATE

BEAM DESCRIPTION: 1ST FLR FRMG - TYP DROPPED CRAWL GRDR BRG 2 LEVEL

B35

PARAMETERS:

L = FT
W = KLF
P = K



ANALYSIS:

$R_{MAX} =$ K $V_D =$ K $< V_{ALL} =$ K ADEQUATE
 $M_{MAX} =$ K-FT $< M_{ALL} =$ K-FT ($C_D=1.0$) ADEQUATE
 $\Delta_{TL} =$ IN. $L/$ $< L/240$ ADEQUATE



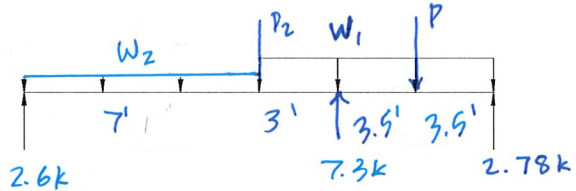
BEAM & HEADER CALCULATIONS

BEAM DESCRIPTION: 1ST FLR FRMG - FLUSH BM 2-SPAN @ BRG ABOVE

B36

PARAMETERS:

L = FT
W₁ = KLF W₂ = 0.15
P₁ = K P₂ = 2.5



ANALYSIS:

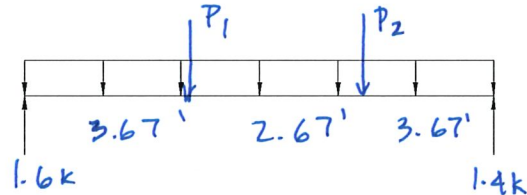
R_{MAX} = K V_D = K < V_{ALL} = K ADEQUATE
M_{MAX} = K-FT < M_{ALL} = K-FT (C_D=1.15) ADEQUATE
Δ_{TL} = IN. L/ < L/240 ADEQUATE

BEAM DESCRIPTION: 1ST FLR FRMG - FLUSH BM @ STAIRS

B37

PARAMETERS:

L = FT
W = KLF
P₁ = K P₂ = 0.2



ANALYSIS:

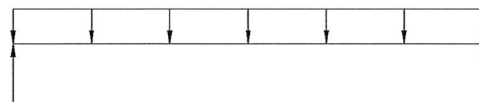
R_{MAX} = K V_D = K < V_{ALL} = K ADEQUATE
M_{MAX} = K-FT < M_{ALL} = K-FT (C_D=1.0) ADEQUATE
Δ_{TL} = IN. L/ < L/240 ADEQUATE

BEAM DESCRIPTION: 2ND FLR FRMG - FLUSH BTM BM @ KITCHEN

B38

PARAMETERS:

L = FT
W = KLF
P = K



ANALYSIS:

R_{MAX} = K V_D = K < V_{ALL} = K ADEQUATE
M_{MAX} = K-FT < M_{ALL} = K-FT (C_D=1.0) ADEQUATE
Δ_{TL} = IN. L/ < L/240 ADEQUATE



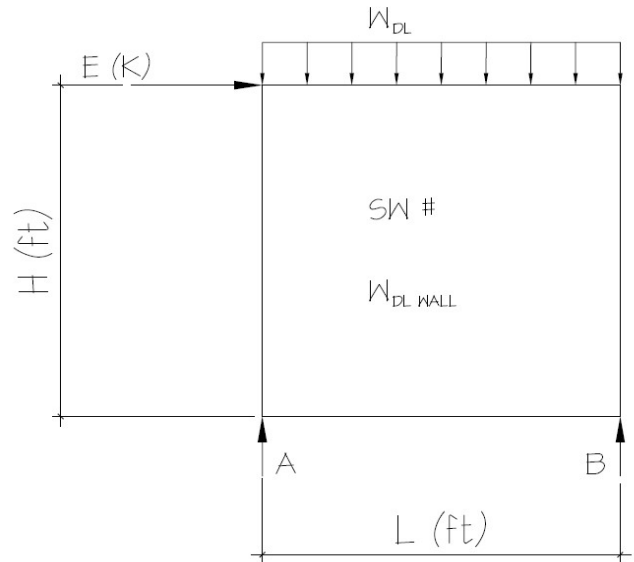
OVERSTRENGTH CALCULATIONS

WALL DESCRIPTION/SW #:

204

PARAMETERS:

- L = 25.8 FT
- H = 9.0 FT
- E = 1.20 K
- W_{DL WALL} = 0.10 KLF
- W_{DL} = 0.094 KLF
- Ω₀ = 2.5 (ASCE TABLE 12.2.1 FOOTNOTE G)
- SDS = 1.152



ANALYSIS:

$E_{MH} = \Omega_0 * E = 3.00 \text{ K}$ $E_v = 0.2 * SDS * DL = 1.151 \text{ K}$
 $E_M = E_{MH} + E_v = 4.151 \text{ K}$
 $E_M = E_{MH} - E_v = 1.849 \text{ K}$

$E_M (\text{MAX}) = \sum M_A = 0 = 4.15(9.0) + 0.194(25.75)(12.875) - R_B(25.75)$ $R_B = 2.5DL + 1.5E$
 $R_A = 2.5DL - 1.5E$
 $E_M (\text{MIN}) = \sum M_A = 0 = 1.85(9.0) + 0.194(25.75)(12.875) - R_B(25.75)$ $R_B = 2.5DL + 0.6E$
 $R_A = 2.5DL - 0.6E$

CHECK BEAMS FOR AXIAL FORCES SHOWN USING LOAD COMBOS PER SECTION 12.4.3.1 (ASD)

ALLOWABLE STRESS PERMITTED TO BE INCREASED BY 1.2

SEE FOLLOWING BEAM
CALCS FOR LOAD
APPLICATION



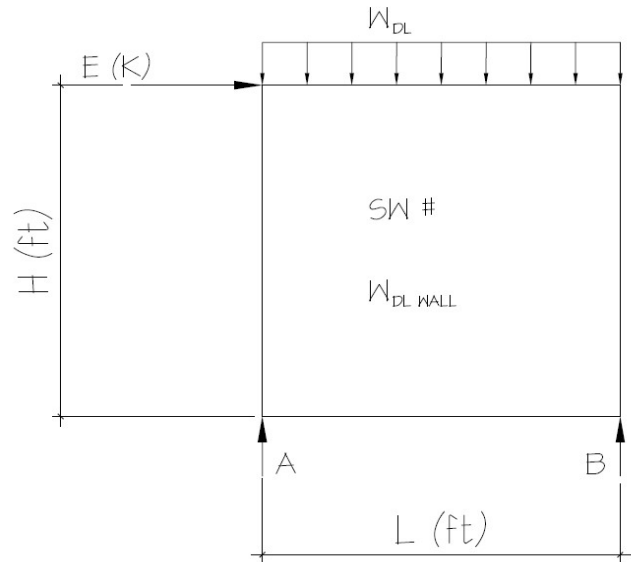
OVERSTRENGTH CALCULATIONS

WALL DESCRIPTION/SW #:

206

PARAMETERS:

- L = 15.5 FT
- H = 9.0 FT
- E = 2.50 K
- W_{DLWALL} = 0.10 KLF
- W_{DL} = 0.034 KLF
- Ω_0 = 2.5 (ASCE TABLE 12.2.1 FOOTNOTE G)
- SDS = 1.152



ANALYSIS:

$$E_{MH} = \Omega_0 * E = 6.25 \text{ K}$$

$$E_v = 0.2 * SDS * DL = 0.479 \text{ K}$$

$$E_M = E_{MH} + E_v = 6.729 \text{ K}$$

$$E_M = E_{MH} - E_v = 5.771 \text{ K}$$

$$E_M (\text{MAX}) = \sum M_A = 0 = 6.73(9.0) + 0.134(15.5)(7.75) - R_B(15.5)$$

$$R_B = 1.0DL + 3.9E$$

$$R_A = 1.0DL - 3.9E$$

$$E_M (\text{MIN}) = \sum M_A = 0 = 5.77(9.0) + 0.134(15.5)(7.75) - R_B(15.5)$$

$$R_B = 1.0DL + 3.4E$$

$$R_A = 1.0DL - 3.4E$$

CHECK BEAMS FOR AXIAL FORCES SHOWN USING LOAD COMBOS PER SECTION 12.4.3.1 (ASD)

ALLOWABLE STRESS PERMITTED TO BE INCREASED BY 1.2

SEE FOLLOWING BEAM
CALCS FOR LOAD
APPLICATION

Wood Beam

Project File: OVERSTRENGTH.ec6

LIC# : KW-06017913, Build:20.22.8.17

MULHERN & KULP STRUCTURAL ENGINEERING INC

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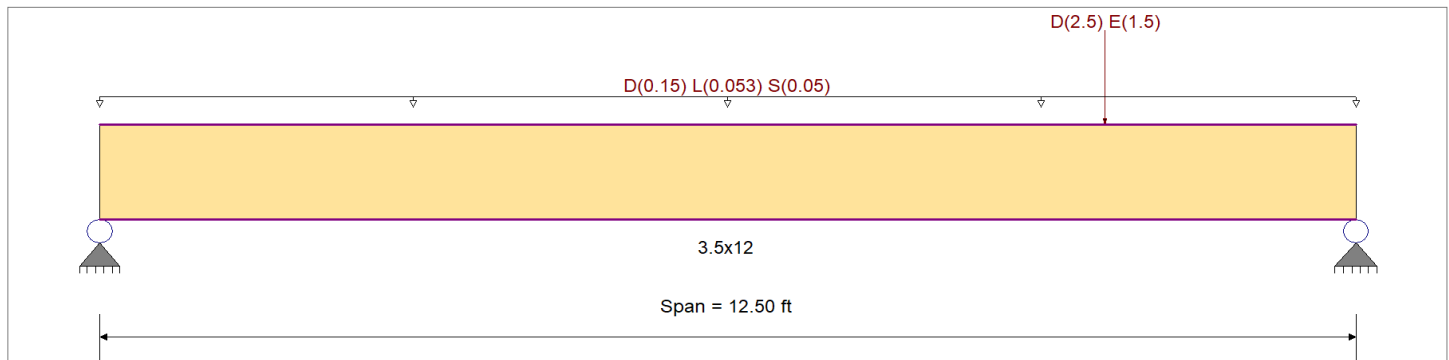
DESCRIPTION: B8 - 2ND FLR FRMG - DROPPED BM @ SIDE COVERED PATIO REAR

CODE REFERENCES

Calculations per NDS 2018, IBC 2018, CBC 2019, ASCE 7-16
Load Combination Set : ASCE 7-16

Material Properties

Analysis Method : Allowable Stress Design	Fb +	2,880.0 psi	E : Modulus of Elasticity	
Load Combination : ASCE 7-16	Fb -	2,220.0 psi	Ebend- xx	1,800.0ksi
	Fc - Prll	1,980.0 psi	Eminbend - xx	950.0ksi
Wood Species : DF/DF	Fc - Perp	780.0 psi	Ebend- yy	1,600.0ksi
Wood Grade : 24F - V4	Fv	318.0 psi	Eminbend - yy	850.0ksi
	Ft	1,320.0 psi	Density	31.210pcf
Beam Bracing : Beam is Fully Braced against lateral-torsional buckling				



Applied Loads

Service loads entered. Load Factors will be applied for calculations.

Beam self weight calculated and added to loading

Uniform Load : D = 0.150, L = 0.0530, S = 0.050, Tributary Width = 1.0 ft, (WALL)

Point Load : D = 2.50, E = 1.50 k @ 10.0 ft, (SW 204)

DESIGN SUMMARY

Design OK

Maximum Bending Stress Ratio	=	0.390 : 1	Maximum Shear Stress Ratio	=	0.355 : 1
Section used for this span		3.5x12	Section used for this span		3.5x12
fb: Actual	=	1,122.42psi	fv: Actual	=	101.50 psi
Fb: Allowable	=	2,880.00psi	Fv: Allowable	=	286.20 psi
Load Combination		+D+L	Load Combination		D Only
Location of maximum on span	=	8.622ft	Location of maximum on span	=	11.542ft
Span # where maximum occurs	=	Span # 1	Span # where maximum occurs	=	Span # 1
Maximum Deflection					
Max Downward Transient Deflection		0.068 in	Ratio =	2216 >=360	Span: 1 : E Only
Max Upward Transient Deflection		0 in	Ratio =	0 <360	n/a
Max Downward Total Deflection		0.310 in	Ratio =	484 >=300	Span: 1 : +1.090D+0.750L+0.750S+0.5250E
Max Upward Total Deflection		0 in	Ratio =	0 <300	n/a

Maximum Forces & Stresses for Load Combinations

Load Combination	Segment Length	Span #	Max Stress Ratios								Moment Values			Shear Values								
			M	V	C _d	C _{F/V}	C _i	C _r	C _m	C _t	C _L	M	fb	F'b	V	fv	F'v					
D Only	Length = 12.50 ft	1	0.387	0.355	0.90	1.000	1.00	1.00	1.00	1.00	1.00	1.00	1.00	7.02	1,002.59	2592.00	0.00	0.00	0.00	2.84	101.50	286.20
+D+L	Length = 12.50 ft	1	0.390	0.351	1.00	1.000	1.00	1.00	1.00	1.00	1.00	1.00	1.00	7.86	1,122.42	2880.00	0.00	0.00	0.00	3.12	111.52	318.00
+D+Lr	Length = 12.50 ft	1	0.278	0.255	1.25	1.000	1.00	1.00	1.00	1.00	1.00	1.00	1.00	7.02	1,002.59	3600.00	0.00	0.00	0.00	2.84	101.50	397.50
+D+S	Length = 12.50 ft	1	0.337	0.303	1.15	1.000	1.00	1.00	1.00	1.00	1.00	1.00	1.00	7.81	1,115.26	3312.00	0.00	0.00	0.00	3.11	110.95	365.70
+D+0.750Lr+0.750L	Length = 12.50 ft	1	0.303	0.274	1.25	1.000	1.00	1.00	1.00	1.00	1.00	1.00	1.00	7.64	1,091.06	3600.00	0.00	0.00	0.00	3.05	109.01	397.50
+D+0.750L+0.750S	Length = 12.50 ft	1	0.357	0.317	1.15	1.000	1.00	1.00	1.00	1.00	1.00	1.00	1.00	8.27	1,181.44	3312.00	0.00	0.00	0.00	3.25	116.10	365.70

Wood Beam

Project File: OVERSTRENGTH.ec6

LIC# : KW-06017913, Build:20.22.8.17

MULHERN & KULP STRUCTURAL ENGINEERING INC

(c) ENERCALC INC 1983-2022

DESCRIPTION: B8 - 2ND FLR FRMG - DROPPED BM @ SIDE COVERED PATIO REAR

Maximum Forces & Stresses for Load Combinations

Load Combination	Segment Length	Span #	Max Stress Ratios								Moment Values			Shear Values			
			M	V	C _d	C _{F/V}	C _i	C _r	C _m	C _t	C _L	M	fb	F'b	V	f _v	F'v
+D+0.60W						1.000	1.00	1.00	1.00	1.00	1.00			0.00	0.00	0.00	0.00
Length = 12.50 ft	1		0.218	0.199	1.60	1.000	1.00	1.00	1.00	1.00	1.00	7.02	1,002.59	4608.00	2.84	101.50	508.80
+1.126D+0.70E						1.000	1.00	1.00	1.00	1.00	1.00			0.00	0.00	0.00	0.00
Length = 12.50 ft	1		0.309	0.284	1.60	1.000	1.00	1.00	1.00	1.00	1.00	9.97	1,424.06	4608.00	4.04	144.29	508.80
+1.126D-0.70E						1.000	1.00	1.00	1.00	1.00	1.00			0.00	0.00	0.00	0.00
Length = 12.50 ft	1		0.188	0.166	1.60	1.000	1.00	1.00	1.00	1.00	1.00	6.05	864.72	4608.00	2.36	84.29	508.80
+D+0.750Lr+0.750L+0.450W						1.000	1.00	1.00	1.00	1.00	1.00			0.00	0.00	0.00	0.00
Length = 12.50 ft	1		0.237	0.214	1.60	1.000	1.00	1.00	1.00	1.00	1.00	7.64	1,091.06	4608.00	3.05	109.01	508.80
+D+0.750L+0.750S+0.450W						1.000	1.00	1.00	1.00	1.00	1.00			0.00	0.00	0.00	0.00
Length = 12.50 ft	1		0.256	0.228	1.60	1.000	1.00	1.00	1.00	1.00	1.00	8.27	1,181.44	4608.00	3.25	116.10	508.80
+1.090D+0.750L+0.750S+0.5						1.000	1.00	1.00	1.00	1.00	1.00			0.00	0.00	0.00	0.00
Length = 12.50 ft	1		0.318	0.290	1.60	1.000	1.00	1.00	1.00	1.00	1.00	10.27	1,467.27	4608.00	4.14	147.73	508.80
+1.090D+0.750L+0.750S-0.5						1.000	1.00	1.00	1.00	1.00	1.00			0.00	0.00	0.00	0.00
Length = 12.50 ft	1		0.236	0.202	1.60	1.000	1.00	1.00	1.00	1.00	1.00	7.62	1,088.19	4608.00	2.88	102.73	508.80
+0.60D+0.60W						1.000	1.00	1.00	1.00	1.00	1.00			0.00	0.00	0.00	0.00
Length = 12.50 ft	1		0.131	0.120	1.60	1.000	1.00	1.00	1.00	1.00	1.00	4.21	601.55	4608.00	1.71	60.90	508.80
+0.470D+0.70E						1.000	1.00	1.00	1.00	1.00	1.00			0.00	0.00	0.00	0.00
Length = 12.50 ft	1		0.167	0.153	1.60	1.000	1.00	1.00	1.00	1.00	1.00	5.38	769.03	4608.00	2.18	77.70	508.80
+0.470D-0.70E						1.000	1.00	1.00	1.00	1.00	1.00			0.00	0.00	0.00	0.00
Length = 12.50 ft	1		0.050	0.035	1.60	1.000	1.00	1.00	1.00	1.00	1.00	1.62	231.56	4608.00	0.50	17.70	508.80

Overall Maximum Deflections

Load Combination	Span	Max. "-" Defl	Location in Span	Load Combination	Max. "+" Defl	Location in Span
+1.090D+0.750L+0.750S+0.5250E	1	0.3096	6.661		0.0000	0.000

Vertical Reactions

Support notation : Far left is #1

Values in KIPS

Load Combination	Support 1	Support 2
Overall MAXimum	2.269	4.377
Overall MINimum	0.300	1.200
D Only	1.494	2.994
+D+L	1.826	3.326
+D+Lr	1.494	2.994
+D+S	1.807	3.307
+D+0.750Lr+0.750L	1.743	3.243
+D+0.750L+0.750S	1.977	3.477
+D+0.60W	1.494	2.994
+1.126D+0.70E	1.893	4.212
+D+0.750Lr+0.750L+0.450W	1.743	3.243
+D+0.750L+0.750S+0.450W	1.977	3.477
+1.090D+0.750L+0.750S+0.5250E	2.269	4.377
+0.60D+0.60W	0.897	1.797
+0.470D+0.70E	0.912	2.247
D Only	1.494	2.994
L Only	0.331	0.331
S Only	0.313	0.313
E Only	0.300	1.200
H Only		

Wood Beam

Project File: OVERSTRENGTH.ec6

LIC# : KW-06017913, Build:20.22.10.25

MULHERN & KULP STRUCTURAL ENGINEERING INC

(c) ENERCALC INC 1983-2022

DESCRIPTION: B10 - 2ND FLR FRMG - DROPPED BM @ SIDE COVERED PATIO FRONT

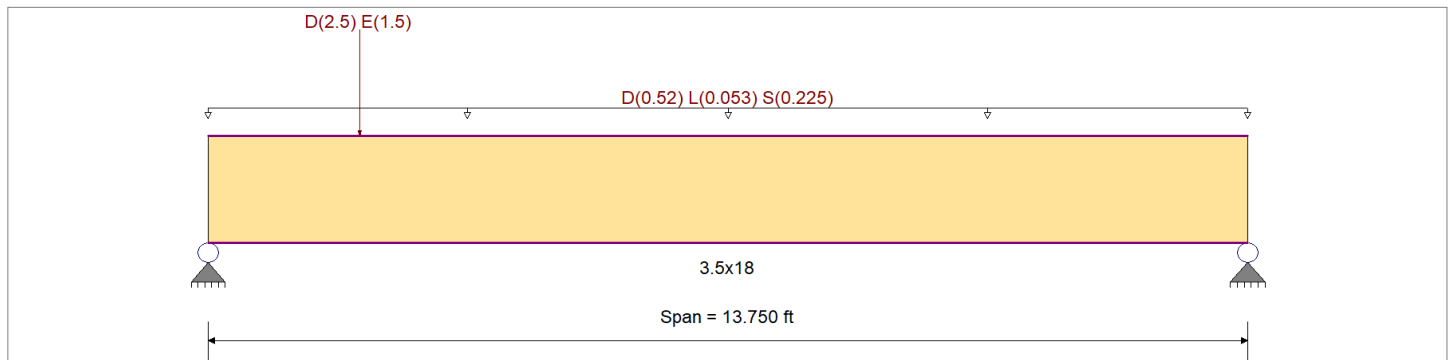
CODE REFERENCES

Calculations per NDS 2018, IBC 2018, CBC 2019, ASCE 7-16

Load Combination Set : ASCE 7-16

Material Properties

Analysis Method : Allowable Stress Design	Fb +	2,880.0 psi	E : Modulus of Elasticity	
Load Combination : ASCE 7-16	Fb -	2,220.0 psi	Ebend- xx	1,800.0ksi
	Fc - Prll	1,980.0 psi	Eminbend - xx	950.0ksi
Wood Species : DF/DF	Fc - Perp	780.0 psi	Ebend- yy	1,600.0ksi
Wood Grade : 24F - V4	Fv	318.0 psi	Eminbend - yy	850.0ksi
	Ft	1,320.0 psi	Density	31.210pcf
Beam Bracing : Beam is Fully Braced against lateral-torsional buckling				



Applied Loads

Service loads entered. Load Factors will be applied for calculations.

Beam self weight calculated and added to loading

Uniform Load : D = 0.520, L = 0.0530, S = 0.2250, Tributary Width = 1.0 ft

Point Load : D = 2.50, E = 1.50 k @ 2.0 ft

DESIGN SUMMARY

Design OK

Maximum Bending Stress Ratio	=	0.393	1	Maximum Shear Stress Ratio	=	0.418	: 1
Section used for this span		3.5x18		Section used for this span		3.5x18	
fb: Actual	=	1,302.61 psi		fv: Actual	=	119.73 psi	
F'b	=	3,312.00 psi		F'v	=	286.20 psi	
Load Combination		+D+S		Load Combination		D Only	
Location of maximum on span	=	6.373ft		Location of maximum on span	=	0.000ft	
Span # where maximum occurs	=	Span # 1		Span # where maximum occurs	=	Span # 1	
Maximum Deflection							
Max Downward Transient Deflection		0.059 in	Ratio =	2775	>=	360	Span: 1 : S Only
Max Upward Transient Deflection		0 in	Ratio =	0	<	360	n/a
Max Downward Total Deflection		0.255 in	Ratio =	647	>=	300	Span: 1 : +1.090D+0.750L+0.750S+0.5250E
Max Upward Total Deflection		0 in	Ratio =	0	<	300	n/a

Maximum Forces & Stresses for Load Combinations

Load Combination	Segment Length	Span #	Max Stress Ratios										Moment Values			Shear Values		
			M	V	CD	CM	C _t	CLx	C _v	C _{fu}	C _i	C _r	M	fb	F'b	V	fv	F'v
D Only															0.0	0.00	0.0	0.0
Length = 13.750 ft	1	0.373	0.418	0.90	1.00	1.00	1.00	1.000	1.00	1.00	1.00	15.24	967.3	2,592.0	5.03	119.7	286.2	
+D+L															0.0	0.00	0.0	0.0
Length = 13.750 ft	1	0.363	0.398	1.00	1.00	1.00	1.00	1.000	1.00	1.00	1.00	16.48	1,046.2	2,880.0	5.32	126.6	318.0	
+D+Lr															0.0	0.00	0.0	0.0
Length = 13.750 ft	1	0.269	0.301	1.25	1.00	1.00	1.00	1.000	1.00	1.00	1.00	15.24	967.3	3,600.0	5.03	119.7	397.5	
+D+S															0.0	0.00	0.0	0.0
Length = 13.750 ft	1	0.393	0.407	1.15	1.00	1.00	1.00	1.000	1.00	1.00	1.00	20.52	1,302.6	3,312.0	6.25	148.8	365.7	
+D+0.750Lr+0.750L															0.0	0.00	0.0	0.0

Wood Beam

Project File: OVERSTRENGTH.ec6

LIC# : KW-06017913, Build:20.22.10.25

MULHERN & KULP STRUCTURAL ENGINEERING INC

(c) ENERCALC INC 1983-2022

DESCRIPTION: B10 - 2ND FLR FRMG - DROPPED BM @ SIDE COVERED PATIO FRONT

Maximum Forces & Stresses for Load Combinations

Load Combination	Max Stress Ratios											Moment Values			Shear Values			
	Segment Length	Span #	M	V	CD	CM	C _t	CLx	C _v	C _{fu}	C _i	C _r	M	fb	F'b	V	fv	F'v
Length = 13.750 ft 1			0.285	0.314	1.25	1.00	1.00	1.00	1.000	1.00	1.00	1.00	16.17	1,026.4	3,600.0	5.24	124.9	397.5
+D+0.750L+0.750S						1.00	1.00	1.00	1.000	1.00	1.00	1.00			0.0	0.00	0.0	0.0
Length = 13.750 ft 1			0.386	0.401	1.15	1.00	1.00	1.00	1.000	1.00	1.00	1.00	20.13	1,278.0	3,312.0	6.16	146.6	365.7
+D+0.60W						1.00	1.00	1.00	1.000	1.00	1.00	1.00			0.0	0.00	0.0	0.0
Length = 13.750 ft 1			0.210	0.235	1.60	1.00	1.00	1.00	1.000	1.00	1.00	1.00	15.24	967.3	4,608.0	5.03	119.7	508.8
+1.126D+0.70E						1.00	1.00	1.00	1.000	1.00	1.00	1.00			0.0	0.00	0.0	0.0
Length = 13.750 ft 1			0.253	0.307	1.60	1.00	1.00	1.00	1.000	1.00	1.00	1.00	18.33	1,163.7	4,608.0	6.56	156.2	508.8
+1.126D-0.70E						1.00	1.00	1.00	1.000	1.00	1.00	1.00			0.0	0.00	0.0	0.0
Length = 13.750 ft 1			0.221	0.223	1.60	1.00	1.00	1.00	1.000	1.00	1.00	1.00	16.02	1,017.2	4,608.0	4.76	113.5	508.8
+D+0.750Lr+0.750L+0.450W						1.00	1.00	1.00	1.000	1.00	1.00	1.00			0.0	0.00	0.0	0.0
Length = 13.750 ft 1			0.223	0.245	1.60	1.00	1.00	1.00	1.000	1.00	1.00	1.00	16.17	1,026.4	4,608.0	5.24	124.9	508.8
+D+0.750L+0.750S+0.450W						1.00	1.00	1.00	1.000	1.00	1.00	1.00			0.0	0.00	0.0	0.0
Length = 13.750 ft 1			0.277	0.288	1.60	1.00	1.00	1.00	1.000	1.00	1.00	1.00	20.13	1,278.0	4,608.0	6.16	146.6	508.8
+1.090D+0.750L+0.750S+0.5						1.00	1.00	1.00	1.000	1.00	1.00	1.00			0.0	0.00	0.0	0.0
Length = 13.750 ft 1			0.308	0.341	1.60	1.00	1.00	1.00	1.000	1.00	1.00	1.00	22.35	1,419.2	4,608.0	7.28	173.4	508.8
+1.090D+0.750L+0.750S-0.5						1.00	1.00	1.00	1.000	1.00	1.00	1.00			0.0	0.00	0.0	0.0
Length = 13.750 ft 1			0.285	0.278	1.60	1.00	1.00	1.00	1.000	1.00	1.00	1.00	20.66	1,311.9	4,608.0	5.94	141.4	508.8
+0.60D+0.60W						1.00	1.00	1.00	1.000	1.00	1.00	1.00			0.0	0.00	0.0	0.0
Length = 13.750 ft 1			0.126	0.141	1.60	1.00	1.00	1.00	1.000	1.00	1.00	1.00	9.14	580.4	4,608.0	3.02	71.8	508.8
+0.470D+0.70E						1.00	1.00	1.00	1.000	1.00	1.00	1.00			0.0	0.00	0.0	0.0
Length = 13.750 ft 1			0.115	0.153	1.60	1.00	1.00	1.00	1.000	1.00	1.00	1.00	8.36	530.9	4,608.0	3.26	77.6	508.8
+0.470D-0.70E						1.00	1.00	1.00	1.000	1.00	1.00	1.00			0.0	0.00	0.0	0.0
Length = 13.750 ft 1			0.083	0.069	1.60	1.00	1.00	1.00	1.000	1.00	1.00	1.00	6.05	384.3	4,608.0	1.47	34.9	508.8

Overall Maximum Deflections

Load Combination	Span	Max. "-" Defl	Location in Span	Load Combination	Max. "+" Defl	Location in Span
+1.090D+0.750L+0.750S+0.5250L	1	0.2548	6.724		0.0000	0.000

Vertical Reactions

Support notation : Far left is #1

Values in KIPS

Load Combination	Support 1	Support 2
Max Upward from all Load Conditions	8.434	5.943
Max Upward from Load Combinations	8.434	5.943
Max Upward from Load Cases	5.805	4.033
D Only	5.805	4.033
+D+L	6.170	4.397
+D+Lr	5.805	4.033
+D+S	7.352	5.579
+D+0.750Lr+0.750L	6.079	4.306
+D+0.750L+0.750S	7.239	5.466
+D+0.60W	5.805	4.033
+1.126D+0.70E	7.434	4.693
+D+0.750Lr+0.750L+0.450W	6.079	4.306
+D+0.750L+0.750S+0.450W	7.239	5.466
+1.090D+0.750L+0.750S+0.5250E	8.434	5.943
+0.60D+0.60W	3.483	2.420
+0.470D+0.70E	3.626	2.048
D Only	5.805	4.033
L Only	0.364	0.364
S Only	1.547	1.547
E Only	1.282	0.218
H Only		

Wood Beam

Project File: OVERSTRENGTH.ec6

LIC# : KW-06017913, Build:20.22.8.17

MULHERN & KULP STRUCTURAL ENGINEERING INC

(c) ENERCALC INC 1983-2022

DESCRIPTION: B30 - 2ND FLR FRMG - FLUSH BTM BM @ GARAGE

CODE REFERENCES

Calculations per NDS 2018, IBC 2018, CBC 2019, ASCE 7-16

Load Combination Set : ASCE 7-16

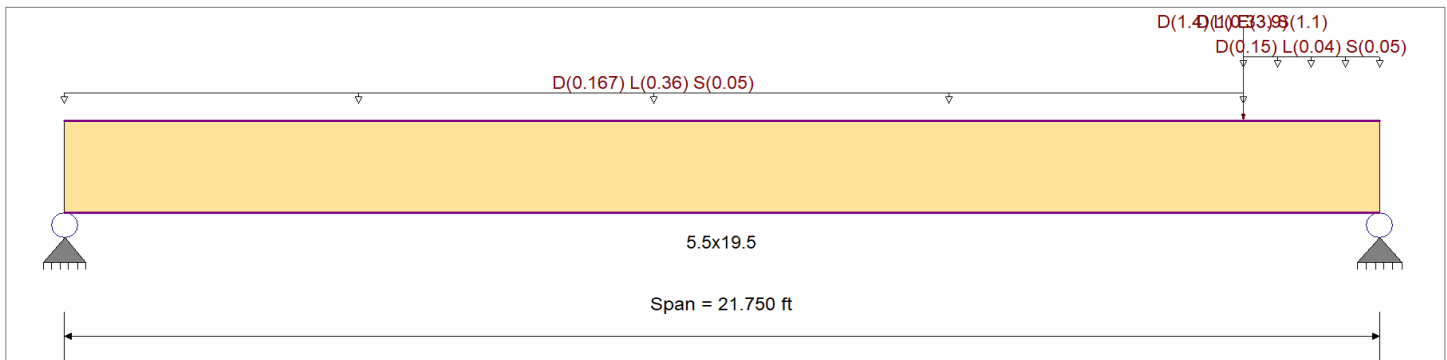
Material Properties

Analysis Method : Allowable Stress Design
Load Combination : ASCE 7-16

Wood Species : DF/DF
Wood Grade : 24F - V4

Beam Bracing : Beam is Fully Braced against lateral-torsional buckling

Fb +	2,880.0 psi	E : Modulus of Elasticity	
Fb -	2,220.0 psi	Ebend- xx	1,800.0ksi
Fc - Prll	1,980.0 psi	Eminbend - xx	950.0ksi
Fc - Perp	780.0 psi	Ebend- yy	1,600.0ksi
Fv	318.0 psi	Eminbend - yy	850.0ksi
Ft	1,320.0 psi	Density	31.210pcf



Applied Loads

Service loads entered. Load Factors will be applied for calculations.

Beam self weight calculated and added to loading

Load for Span Number 1

Uniform Load : D = 0.1670, L = 0.360, S = 0.050 k/ft, Extent = 0.0 --> 19.50 ft, Tributary Width = 1.0 ft, (DECK)

Uniform Load : D = 0.150, L = 0.040, S = 0.050 k/ft, Extent = 19.50 --> 21.750 ft, Tributary Width = 1.0 ft, (WALL ABOVE)

Point Load : D = 1.40, L = 0.330, S = 1.10 k @ 19.50 ft, (B29)

Point Load : D = 1.0, E = 3.90 k @ 19.50 ft, (SW 206)

DESIGN SUMMARY

Design OK

Maximum Bending Stress Ratio	=	0.447 : 1	Maximum Shear Stress Ratio	=	0.324 : 1
Section used for this span		5.5x19.5	Section used for this span		5.5x19.5
fb: Actual	=	1,213.06psi	fv: Actual	=	103.13 psi
Fb: Allowable	=	2,714.67psi	Fv: Allowable	=	318.00 psi
Load Combination		+D+L	Load Combination		+D+L
Location of maximum on span	=	11.351 ft	Location of maximum on span	=	20.162 ft
Span # where maximum occurs	=	Span # 1	Span # where maximum occurs	=	Span # 1
Maximum Deflection					
Max Downward Transient Deflection	0.297 in	Ratio = 877 >=360	Span: 1 : L Only		
Max Upward Transient Deflection	0 in	Ratio = 0 <360	n/a		
Max Downward Total Deflection	0.528 in	Ratio = 494 >=300	Span: 1 : +1.090D+0.750L+0.750S+0.5250E		
Max Upward Total Deflection	0 in	Ratio = 0 <300	n/a		

Maximum Forces & Stresses for Load Combinations

Load Combination	Segment Length	Span #	Max Stress Ratios								Moment Values			Shear Values						
			M	V	C _d	C _{F/V}	C _i	C _r	C _m	C _t	C _L	M	fb	F'b	V	fv	F'v			
D Only	Length = 21.750 ft	1	0.199	0.191	0.90	0.943	1.00	1.00	1.00	1.00	1.00	1.00	1.00	14.09	484.99	2443.20	0.00	0.00	0.00	0.00
+D+L	Length = 21.750 ft	1	0.447	0.324	1.00	0.943	1.00	1.00	1.00	1.00	1.00	1.00	1.00	35.24	1,213.06	2714.67	0.00	0.00	0.00	0.00
+D+Lr	Length = 21.750 ft	1	0.143	0.138	1.25	0.943	1.00	1.00	1.00	1.00	1.00	1.00	1.00	14.09	484.99	3393.33	0.00	0.00	0.00	0.00
+D+S	Length = 21.750 ft	1	0.203	0.205	1.15	0.943	1.00	1.00	1.00	1.00	1.00	1.00	1.00	18.39	633.19	3121.86	0.00	0.00	0.00	0.00
+D+0.750Lr+0.750L						0.943	1.00	1.00	1.00	1.00	1.00	1.00	1.00			0.00	0.00	0.00	0.00	0.00

Wood Beam

Project File: OVERSTRENGTH.ec6

LIC# : KW-06017913, Build:20.22.8.17

MULHERN & KULP STRUCTURAL ENGINEERING INC

(c) ENERCALC INC 1983-2022

DESCRIPTION: B30 - 2ND FLR FRMG - FLUSH BTM BM @ GARAGE

Maximum Forces & Stresses for Load Combinations

Load Combination	Segment Length	Span #	Max Stress Ratios								Moment Values			Shear Values		
			M	V	C _d	C _{F/V}	C _i	C _r	C _m	C _t	C _L	M	fb	F'b	V	fv
Length = 21.750 ft	1	0.304	0.229	1.25	0.943	1.00	1.00	1.00	1.00	1.00	29.93	1,030.52	3393.33	6.51	91.02	397.50
+D+0.750L+0.750S					0.943	1.00	1.00	1.00	1.00	1.00			0.00	0.00	0.00	0.00
Length = 21.750 ft	1	0.365	0.290	1.15	0.943	1.00	1.00	1.00	1.00	1.00	33.12	1,140.33	3121.86	7.60	106.23	365.70
+D+0.60W					0.943	1.00	1.00	1.00	1.00	1.00			0.00	0.00	0.00	0.00
Length = 21.750 ft	1	0.112	0.107	1.60	0.943	1.00	1.00	1.00	1.00	1.00	14.09	484.99	4343.46	3.91	54.68	508.80
+1.126D+0.70E					0.943	1.00	1.00	1.00	1.00	1.00			0.00	0.00	0.00	0.00
Length = 21.750 ft	1	0.154	0.188	1.60	0.943	1.00	1.00	1.00	1.00	1.00	19.49	670.84	4343.46	6.85	95.80	508.80
+1.126D-0.70E					0.943	1.00	1.00	1.00	1.00	1.00			0.00	0.00	0.00	0.00
Length = 21.750 ft	1	0.100	0.055	1.60	0.943	1.00	1.00	1.00	1.00	1.00	12.61	434.19	4343.46	1.98	27.75	508.80
+D+0.750Lr+0.750L+0.450W					0.943	1.00	1.00	1.00	1.00	1.00			0.00	0.00	0.00	0.00
Length = 21.750 ft	1	0.237	0.179	1.60	0.943	1.00	1.00	1.00	1.00	1.00	29.93	1,030.52	4343.46	6.51	91.02	508.80
+D+0.750L+0.750S+0.450W					0.943	1.00	1.00	1.00	1.00	1.00			0.00	0.00	0.00	0.00
Length = 21.750 ft	1	0.263	0.209	1.60	0.943	1.00	1.00	1.00	1.00	1.00	33.12	1,140.33	4343.46	7.60	106.23	508.80
+1.090D+0.750L+0.750S+0.5					0.943	1.00	1.00	1.00	1.00	1.00			0.00	0.00	0.00	0.00
Length = 21.750 ft	1	0.292	0.269	1.60	0.943	1.00	1.00	1.00	1.00	1.00	36.88	1,269.65	4343.46	9.78	136.83	508.80
+1.090D+0.750L+0.750S-0.5z					0.943	1.00	1.00	1.00	1.00	1.00			0.00	0.00	0.00	0.00
Length = 21.750 ft	1	0.254	0.168	1.60	0.943	1.00	1.00	1.00	1.00	1.00	31.98	1,101.09	4343.46	6.11	85.48	508.80
+0.60D+0.60W					0.943	1.00	1.00	1.00	1.00	1.00			0.00	0.00	0.00	0.00
Length = 21.750 ft	1	0.067	0.064	1.60	0.943	1.00	1.00	1.00	1.00	1.00	8.45	291.00	4343.46	2.35	32.81	508.80
+0.470D+0.70E					0.943	1.00	1.00	1.00	1.00	1.00			0.00	0.00	0.00	0.00
Length = 21.750 ft	1	0.083	0.118	1.60	0.943	1.00	1.00	1.00	1.00	1.00	10.50	361.62	4343.46	4.28	59.93	508.80
+0.470D-0.70E					0.943	1.00	1.00	1.00	1.00	1.00			0.00	0.00	0.00	0.00
Length = 21.750 ft	1	0.029	0.026	1.60	0.943	1.00	1.00	1.00	1.00	1.00	3.63	124.98	4343.46	0.93	13.05	508.80

Overall Maximum Deflections

Load Combination	Span	Max. "-" Defl	Location in Span	Load Combination	Max. "+" Defl	Location in Span
+1.090D+0.750L+0.750S+0.5250I	1	0.5280	11.193		0.0000	0.000

Vertical Reactions

Support notation : Far left is #1

Values in KIPS

Load Combination	Support 1	Support 2
Overall MAXimum	6.227	10.190
Overall MINimum	0.403	3.497
D Only	2.315	4.184
+D+L	6.227	7.712
+D+Lr	2.315	4.184
+D+S	2.973	5.714
+D+0.750Lr+0.750L	5.249	6.830
+D+0.750L+0.750S	5.742	7.978
+D+0.60W	2.315	4.184
+1.126D+0.70E	2.889	7.159
+D+0.750Lr+0.750L+0.450W	5.249	6.830
+D+0.750L+0.750S+0.450W	5.742	7.978
+1.090D+0.750L+0.750S+0.5250E	6.162	10.190
+0.60D+0.60W	1.389	2.511
+0.470D+0.70E	1.371	4.414
D Only	2.315	4.184
L Only	3.912	3.528
S Only	0.658	1.530
E Only	0.403	3.497
H Only		

JAYMARC HOMES
SPRING RESIDENCE

MERCER ISLAND, WA

SHEAR WALL CALCULATIONS - WIND

REVIEWED BY: RJZ

NOVEMBER 1, 2022

PARAMETERS:

SINGLE FAMILY HOME

DESIGN WIND SPEED: 100 MPH

WIND EXPOSURE CATEGORY: C

SEISMIC DESIGN CATEGORY: D

CODE & DESIGN STANDARD: 2018 IBC CH. 1609, ASCE 7-16 CH. 26-30



MULHERN+KULP
RESIDENTIAL STRUCTURAL ENGINEERING

WIND DESIGN SUMMARY PER ASCE 7-16

PARAMETERS:		ROOF GEOMETRY:		BUILDING GEOMETRY:	
WIND SPEED	100	TRANS. ROOF PITCH	6.0 : 12	LENGTH	95 FT
EXPOSURE CATEGORY	C	LONG. ROOF PITCH	7.0 : 12	WIDTH	45 FT
RISK CATEGORY	II	MEAN ROOF HEIGHT, H	24.00 FT	NUMBER OF STORIES	2
WIND DIRECTIONALITY FACTOR, K_D	0.85				
TOPOGRAPHIC FACTOR, K_{ZT}	1.60				
GUST FACTOR, G	0.85				
GROUND ELEV. ABOVE SEA LEVEL [FT]	0				
DESIGN TYPE	ASD 0.60				

TRANSVERSE DIRECTION (PERPENDICULAR TO MAIN RIDGE LINE)						
DIAPHRAGM LEVEL	FLOOR-TO-FLOOR HEIGHT	Roof Surface	SECTION			sq ft
			A	O	B	
2	9 FT	Roof Surface	0	300	0	sq ft
		Wall surface	0	450	0	sq ft
1	11.5 FT	Roof Surface	0	75	0	sq ft
		Wall surface	0	930	0	sq ft
FND		Roof Surface	0	0	0	sq ft
		Wall surface	0	0	0	sq ft

TRIBUTARY DESIGN AREAS:				
DIAPHRAGM LEVEL	FLOOR-TO-FLOOR HEIGHT	SECTION		
		A	O	B
2	9 FT	0	300	0
1	11.5 FT	0	75	0
FND		0	0	0

TRIBUTARY DESIGN LOADS: (0.6W)				
DIAPHRAGM LEVEL	FLOOR-TO-FLOOR HEIGHT	SECTION		
		A	O	B
2	9 FT	0.00	13.49	0.00
		0.00	13.49	0.00
1	11.5 FT	0.00	19.94	0.00
		0.00	33.43	0.00
FND		0.00	0.00	0.00
		0.00	33.43	0.00

LONGITUDINAL DIRECTION (PARALLEL TO MAIN RIDGE LINE)						
DIAPHRAGM LEVEL	FLOOR-TO-FLOOR HEIGHT	Roof Surface	SECTION			sq ft
			A	O	B	
2	9 FT	Roof Surface	0	150	0	sq ft
		Wall surface	0	278	0	sq ft
1	11.5 FT	Roof Surface	0	0	0	sq ft
		Wall surface	0	495	0	sq ft
FND		Roof Surface	0	0	0	sq ft
		Wall surface	0	0	0	sq ft

TRIBUTARY DESIGN AREAS:				
DIAPHRAGM LEVEL	FLOOR-TO-FLOOR HEIGHT	SECTION		
		A	O	B
2	9 FT	0	150	0
1	11.5 FT	0	0	0
FND		0	0	0

TRIBUTARY DESIGN LOADS: (0.6W)				
DIAPHRAGM LEVEL	FLOOR-TO-FLOOR HEIGHT	SECTION		
		A	O	B
2	9 FT	0.00	7.19	0.00
		0.00	7.19	0.00
1	11.5 FT	0.00	8.39	0.00
		0.00	15.58	0.00
FND		0.00	0.00	0.00
		0.00	15.58	0.00

plan name: ---
 marketing name: ---
 plan number: ---
 mark sys. number: ---

Conditions not specifically represented graphically or in writing or which conflict with the current International Residential Code (IRC), or those of the local municipality then the current standards and requirements of each respectively shall govern.

The drawings in this set are instruments of service and shall remain the property of JayMarc Homes, LLC.

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11.22.22
 Submittal Date

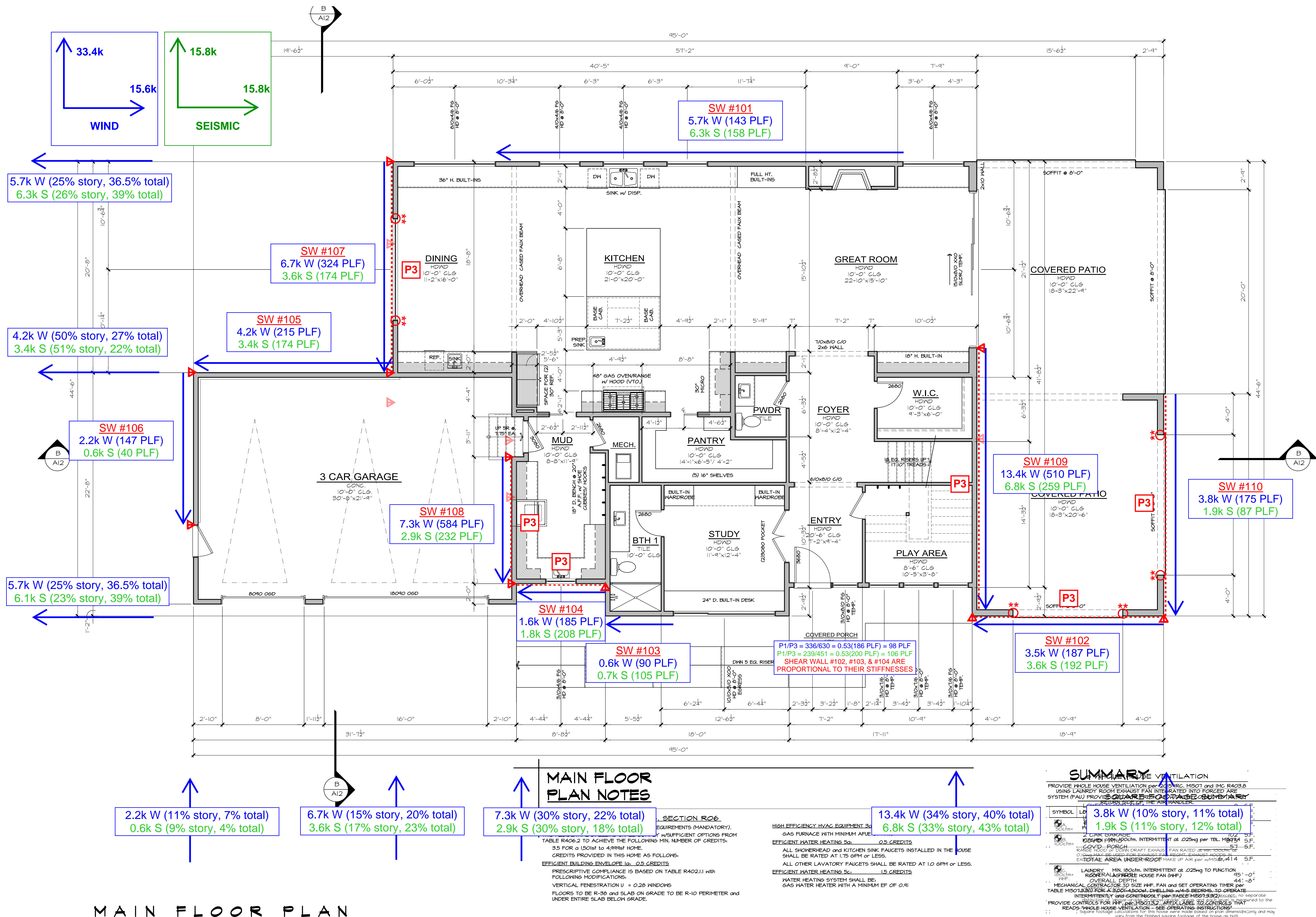
Sheet Title/Description
 JAYMARC HOMES
 Design Firm

R.K.N.
 Drawn by:

Checked by:

Primary Scale

A5
 of .



MAIN FLOOR PLAN NOTES

SECTION R06
 EQUIPMENTS (MANDATORY).
 EQUIPMENTS (OPTIONAL).
 TABLE R406.2 TO ACHIEVE THE FOLLOWING MIN. NUMBER OF CREDITS:
 3.5 FOR A 1501sf to 4,999sf HOME.
 CREDITS PROVIDED IN THIS HOME AS FOLLOWS:
 EFFICIENT BUILDING ENVELOPE (a) .05 CREDITS
 PRESCRIPTIVE COMPLIANCE IS BASED ON TABLE R402.1.1 WITH FOLLOWING MODIFICATIONS:
 VERTICAL FENESTRATION U = 0.29 WINDOWS
 FLOORS TO BE R-39 and SLAB ON GRADE TO BE R-10 PERIMETER and UNDER ENTIRE SLAB BELOW GRADE.

HIGH EFFICIENCY HVAC EQUIPMENT 3
 GAS FURNACE WITH MINIMUM AFUE 80% 1
 EFFICIENT WATER HEATING 5a) 0.5 CREDITS
 ALL SHOWERHEAD AND KITCHEN SINK FAUCETS INSTALLED IN THE HOUSE SHALL BE RATED AT 1.75 GPM or LESS.
 ALL OTHER LAVATORY FAUCETS SHALL BE RATED AT 1.0 GPM or LESS.
 EFFICIENT WATER HEATING 5b) 1.5 CREDITS
 WATER HEATING SYSTEM SHALL BE GAS WATER HEATER WITH A MINIMUM EF OF 0.91

SUMMARY

SYMBOL	LOAD	DESCRIPTION	STRENGTH
SW #101	WIND	5.7k W (143 PLF) 6.3k S (158 PLF)	
SW #102	WIND	3.5k W (187 PLF) 3.6k S (192 PLF)	
SW #103	WIND	0.6k W (90 PLF) 0.7k S (105 PLF)	
SW #104	WIND	1.6k W (185 PLF) 1.8k S (208 PLF)	
SW #105	WIND	4.2k W (215 PLF) 3.4k S (174 PLF)	
SW #106	WIND	2.2k W (147 PLF) 0.6k S (40 PLF)	
SW #107	WIND	6.7k W (324 PLF) 3.6k S (174 PLF)	
SW #108	WIND	7.3k W (584 PLF) 2.9k S (232 PLF)	
SW #109	WIND	13.4k W (510 PLF) 6.8k S (259 PLF)	
SW #110	WIND	3.8k W (175 PLF) 1.9k S (87 PLF)	
SW #111	WIND	2.2k W (11% story, 7% total) 0.6k S (9% story, 4% total)	
SW #112	WIND	6.7k W (15% story, 20% total) 3.6k S (17% story, 23% total)	
SW #113	WIND	7.3k W (30% story, 22% total) 2.9k S (30% story, 18% total)	
SW #114	WIND	13.4k W (34% story, 40% total) 6.8k S (33% story, 43% total)	

Issue	Issue Date	By	Description

Spring Residence
4740 W. Mercer Way
Mercer Island, WA.

Job Number: Spring

plan name:	-
marketing name:	-
plan number:	-
mark sys. number:	-

Conditions not specifically represented graphically or in writing or which conflict with the current International Residential Code (IRC), or those of the local municipality and then the current standards and requirements of each respectively shall govern.

The drawings in this set are instruments of service and shall remain the property of JayMarc Homes, LLC.

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11.22.22
Submission Date

Sheet Title/Description

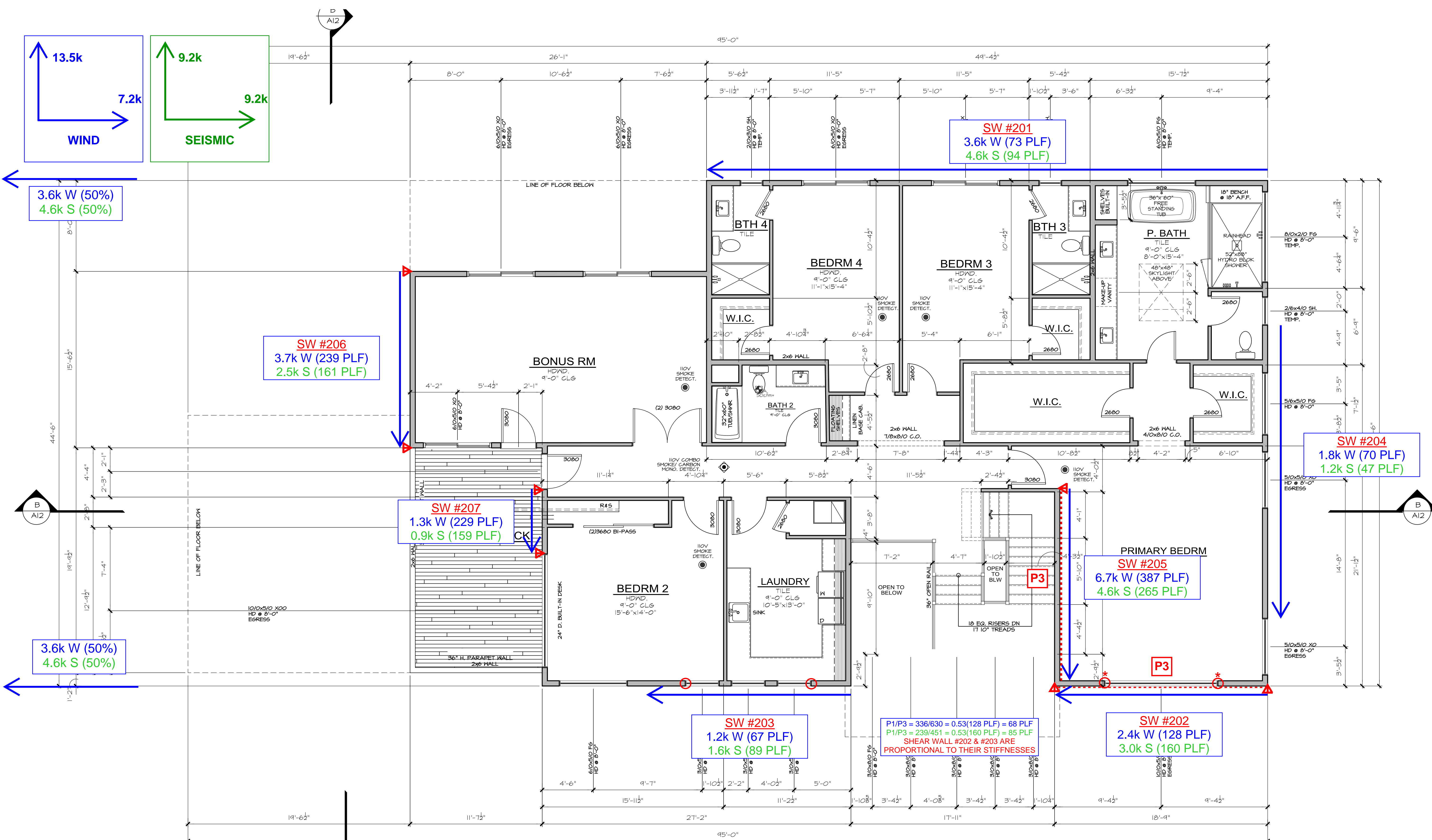
JAYMARC HOMES
Design Firm

R.K.N.
Drawn by:

Checked by:

Primary Scale

A7
of .



UPPER FLOOR PLAN NOTES:

PLAN SPECIFIC 2015 NSEC. SECT. R406.2 ADDITIONAL ENERGY EFFICIENCY REQUIREMENT THIS RESIDENTIAL DWELLING SHALL COMPLY WITH SUFFICIENT TABLE R406.2 TO ACHIEVE THE FOLLOWING MIN. NUMBER CREDITS PROVIDED IN THIS HOME AS FOLLOWS:
EFFICIENT BUILDING ENVELOPE 1a: 0.5 CREDITS
PRESCRIPTIVE COMPLIANCE IS BASED ON TABLE R402.1.1 WITH FOLLOWING MODIFICATIONS:
VERTICAL PENETRATION U = 0.28 WINDOWS
FLOORS TO BE R-38 and SLAB ON GRADE TO BE R-10 PERIMETER and UNDER ENTIRE SLAB BELOW GRADE.

6.2 ADDITIONAL ENERGY EFFICIENCY REQUIREMENTS (MANDATORY).
1. EFFICIENCY HVAC EQUIPMENT 3a: 1.0 CREDITS
2. GAS FURNACE WITH MINIMUM AFUE OF 94%
EFFICIENT WATER HEATING 5a: 0.5 CREDITS
ALL SHOWERHEAD and KITCHEN SINK FAUCETS INSTALLED IN THE HOUSE SHALL BE RATED AT 1.75 GPM or LESS.
ALL OTHER LAVATORY FAUCETS SHALL BE RATED AT 1.0 GPM or LESS.
EFFICIENT WATER HEATING 5c: 1.5 CREDITS
WATER HEATING SYSTEM SHALL BE:
GAS WATER HEATER WITH A MINIMUM EF OF 0.91

WHOLE HOUSE VENTILATION

PROVIDE WHOLE HOUSE VENTILATION per 2015 IRC, M507 and IMC R403.8 USING LAUNDRY ROOM EXHAUST FAN INTEGRATED INTO FORCED AIR SYSTEM (FAU) PROVIDE OUTDOOR FRESH AIR W/DUCTS CONNECTED TO THE RETURN SIDE OF THE AIR HANDLER.

SYMBOL	LOCATION	MIN. FAN REQUIREMENTS (ALL FANS VENT TO OUTSIDE)
⊙	BATH # POWDER ROOM	Min. 50cfm, INTERMITTENT at .025hg per TABLE M507.4
⊙	KITCHEN	Min. 100cfm, INTERMITTENT at .025hg per TBL. M507.4
⊙	LAUNDRY ROOM	MIN. 180cfm, INTERMITTENT at .025hg TO FUNCTION AS WHOLE HOUSE FAN (WHF)

MECHANICAL CONTRACTOR TO SIZE WHF, FAN and SET OPERATING TIMER per TABLE M507.3(3) FOR A 3001-4500H. DWELLING w/4-5 BEDRMS. TO OPERATE INTERMITTENTLY and CONTINUOUSLY per TABLE M507.3(2)
PROVIDE CONTROLS FOR WHF, per M507.3.2 AFFIX LABEL TO CONTROLS THAT READS "WHOLE HOUSE VENTILATION - SEE OPERATING INSTRUCTIONS"

SUMMARY

SQUARE FOOTAGE SUMMARY	
LOWER FLOOR AREA	702 S.F.
MAIN FLOOR AREA	915 S.F.
UPPER FLOOR AREA	57 S.F.
TOTAL CONDITIONED AREA	1674 S.F.
2 CAR GARAGE	702 S.F.
COVID PATIO	57 S.F.
COVID PORCH	57 S.F.
TOTAL AREA UNDER ROOF	6,414 S.F.
OVERALL WIDTH	45'-0"
OVERALL DEPTH	44'-8"

Updated: 12.09.20
Method for Calculating Square Footage - ANSI Z765-2019 excludes no separate distinction of above-grade or below-grade areas and each level is measured to the outside of studs not the exterior finished surface.
Square footage calculations for this house were made based on plan dimensions only and may vary from square footage shown on other house drawings.

UPPER FLOOR PLAN

1/4" = 1'-0"



SHEARWALL DESIGN SUMMARY

SHEARWALL 201: 2ND - REAR EXT BED 4 TO PRIMARY BATH

SHEARWALL PROPERTIES:

WALL HEIGHT, H FT. MAX WALL OPENING HT, H_c FT.
WALL LENGTH, L FT. QUALIFYING WALL LENGTH, L FT. SHEARWALL ASSEMBLY

CAPACITY EVALUATION:

TOTAL SHEAR LOAD ON WALL LBS < ALLOWABLE SHEARWALL CAPACITY LBS

SHEARWALL ASSEMBLY SPECIFICATION

P1 - 1-SIDE 7/16" OSB
FASTENED W/ 8D NAILS AT 6"O.C. PANEL EDGES & 12"O.C. PANEL FIELD - EDGES BLOCKED
ADEQUATE

OVERTURNING EVALUATION:

RESISTIVE DL PLF OVERTURNING MOMENT K-FT HOLD DOWN DESIGN LOAD LBS
DL AT ENDS OF WALL LBS RESISTIVE MOMENT K-FT HOLDOWN CAPACITY LBS

HOLD-DOWN SPECIFICATION

NO HOLDOWN REQUIRED

SHEARWALL 202: 2ND - FRONT EXT PRIMARY BEDROOM

SHEARWALL PROPERTIES:

WALL HEIGHT, H FT. MAX WALL OPENING HT, H_c FT.
WALL LENGTH, L FT. QUALIFYING WALL LENGTH, L FT. SHEARWALL ASSEMBLY

CAPACITY EVALUATION:

TOTAL SHEAR LOAD ON WALL LBS < ALLOWABLE SHEARWALL CAPACITY LBS

SHEARWALL ASSEMBLY SPECIFICATION

P3 - 1-SIDE 7/16" OSB
FASTENED W/ 8D NAILS AT 3"O.C. PANEL EDGES & 12"O.C. PANEL FIELD - EDGES BLOCKED
ADEQUATE

OVERTURNING EVALUATION:

RESISTIVE DL PLF OVERTURNING MOMENT K-FT HOLD DOWN DESIGN LOAD LBS
DL AT ENDS OF WALL LBS RESISTIVE MOMENT K-FT HOLDOWN CAPACITY LBS

HOLD-DOWN SPECIFICATION

SIMPSON GS16 STRAP TIE (14" END LENGTH)



SHEARWALL DESIGN SUMMARY

SHEARWALL 203: 2ND - FRONT EXT BED 2 / LAUNDRY

SHEARWALL PROPERTIES:

WALL HEIGHT, H	<input type="text" value="9.0"/>	FT.	MAX WALL OPENING HT, H _c	<input type="text" value="5.0"/>	FT.		
WALL LENGTH, L	<input type="text" value="18.0"/>	FT.	QUALIFYING WALL LENGTH, L	<input type="text" value="7.0"/>	FT.	SHEARWALL ASSEMBLY	<input type="text" value="P1"/>

CAPACITY EVALUATION:

TOTAL SHEAR LOAD ON WALL	<input type="text" value="1200"/>	LBS	<	ALLOWABLE SHEARWALL CAPACITY	<input type="text" value="2351"/>	LBS
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SHEARWALL ASSEMBLY SPECIFICATION

P1 - 1-SIDE 7/16" OSB
FASTENED W/ 8D NAILS AT 6"O.C. PANEL EDGES & 12"O.C. PANEL FIELD - EDGES BLOCKED
ADEQUATE

OVERTURNING EVALUATION:

RESISTIVE DL	<input type="text" value="134"/>	PLF	OVERTURNING MOMENT	<input type="text" value="10.8"/>	K-FT	HOLD DOWN DESIGN LOAD	<input type="text" value="0"/>	LBS
DL AT ENDS OF WALL	<input type="text" value="1200"/>	LBS	RESISTIVE MOMENT	<input type="text" value="20.0"/>	K-FT	HOLD DOWN CAPACITY	<input type="text" value="0"/>	LBS

HOLD-DOWN SPECIFICATION

NO HOLDOWN REQUIRED

SHEARWALL 204: 2ND - SIDE EXT PRIMARY BEDROOM / BATH

SHEARWALL PROPERTIES:

WALL HEIGHT, H	<input type="text" value="9.0"/>	FT.	MAX WALL OPENING HT, H _c	<input type="text" value="5.0"/>	FT.		
WALL LENGTH, L	<input type="text" value="25.8"/>	FT.	QUALIFYING WALL LENGTH, L	<input type="text" value="13.8"/>	FT.	SHEARWALL ASSEMBLY	<input type="text" value="P1"/>

CAPACITY EVALUATION:

TOTAL SHEAR LOAD ON WALL	<input type="text" value="1800"/>	LBS	<	ALLOWABLE SHEARWALL CAPACITY	<input type="text" value="4617"/>	LBS
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SHEARWALL ASSEMBLY SPECIFICATION

P1 - 1-SIDE 7/16" OSB
FASTENED W/ 8D NAILS AT 6"O.C. PANEL EDGES & 12"O.C. PANEL FIELD - EDGES BLOCKED
ADEQUATE

OVERTURNING EVALUATION:

RESISTIVE DL	<input type="text" value="194"/>	PLF	OVERTURNING MOMENT	<input type="text" value="16.2"/>	K-FT	HOLD DOWN DESIGN LOAD	<input type="text" value="0"/>	LBS
DL AT ENDS OF WALL	<input type="text" value="50"/>	LBS	RESISTIVE MOMENT	<input type="text" value="30.2"/>	K-FT	HOLD DOWN CAPACITY	<input type="text" value="0"/>	LBS

HOLD-DOWN SPECIFICATION

NO HOLDOWN REQUIRED



SHEARWALL DESIGN SUMMARY

SHEARWALL 205: 2ND - SIDE INT PRIMARY BEDROOM

SHEARWALL PROPERTIES:

WALL HEIGHT, H FT. MAX WALL OPENING HT, H_c FT.
WALL LENGTH, L FT. QUALIFYING WALL LENGTH, L FT. SHEARWALL ASSEMBLY

CAPACITY EVALUATION:

TOTAL SHEAR LOAD ON WALL LBS < ALLOWABLE SHEARWALL CAPACITY LBS

SHEARWALL ASSEMBLY SPECIFICATION

P3 - 1-SIDE 7/16" OSB
FASTENED W/ 8D NAILS AT 3"O.C. PANEL EDGES & 12"O.C. PANEL FIELD - EDGES BLOCKED
ADEQUATE

OVERTURNING EVALUATION:

RESISTIVE DL PLF OVERTURNING MOMENT K-FT HOLD DOWN DESIGN LOAD LBS
DL AT ENDS OF WALL LBS RESISTIVE MOMENT K-FT HOLD DOWN CAPACITY LBS

HOLD-DOWN SPECIFICATION

SIMPSON MSTC66 STRAP TIE (20" END LENGTH)

SHEARWALL 206: 2ND - SIDE EXT BONUS

SHEARWALL PROPERTIES:

WALL HEIGHT, H FT. MAX WALL OPENING HT, H_c FT.
WALL LENGTH, L FT. QUALIFYING WALL LENGTH, L FT. SHEARWALL ASSEMBLY

CAPACITY EVALUATION:

TOTAL SHEAR LOAD ON WALL LBS < ALLOWABLE SHEARWALL CAPACITY LBS

SHEARWALL ASSEMBLY SPECIFICATION

P1 - 1-SIDE 7/16" OSB
FASTENED W/ 8D NAILS AT 6"O.C. PANEL EDGES & 12"O.C. PANEL FIELD - EDGES BLOCKED
ADEQUATE

OVERTURNING EVALUATION:

RESISTIVE DL PLF OVERTURNING MOMENT K-FT HOLD DOWN DESIGN LOAD LBS
DL AT ENDS OF WALL LBS RESISTIVE MOMENT K-FT HOLD DOWN CAPACITY LBS

HOLD-DOWN SPECIFICATION

SIMPSON GS16 STRAP TIE (14" END LENGTH)



SHEARWALL DESIGN SUMMARY

SHEARWALL 207: 2ND - SIDE EXT BED 2

SHEARWALL PROPERTIES:

WALL HEIGHT, H	<input type="text" value="9.0"/>	FT.	MAX WALL OPENING HT, H _c	<input type="text" value="0.0"/>	FT.		
WALL LENGTH, L	<input type="text" value="5.7"/>	FT.	QUALIFYING WALL LENGTH, L	<input type="text" value="5.7"/>	FT.	SHEARWALL ASSEMBLY	<input type="text" value="P1"/>

CAPACITY EVALUATION:

TOTAL SHEAR LOAD ON WALL	<input type="text" value="1300"/>	LBS	<	ALLOWABLE SHEARWALL CAPACITY	<input type="text" value="1904"/>	LBS
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SHEARWALL ASSEMBLY SPECIFICATION

P1 - 1-SIDE 7/16" OSB
FASTENED W/ 8D NAILS AT 6"O.C. PANEL EDGES & 12"O.C. PANEL FIELD - EDGES BLOCKED
ADEQUATE

OVERTURNING EVALUATION:

RESISTIVE DL	<input type="text" value="150"/>	PLF	OVERTURNING MOMENT	<input type="text" value="11.7"/>	K-FT	HOLD DOWN DESIGN LOAD	<input type="text" value="1683"/>	LBS
DL AT ENDS OF WALL	<input type="text" value="400"/>	LBS	RESISTIVE MOMENT	<input type="text" value="2.2"/>	K-FT	HOLD DOWN CAPACITY	<input type="text" value="1705"/>	LBS

HOLD-DOWN SPECIFICATION

SIMPSON CS16 STRAP TIE (14" END LENGTH)

SHEARWALL 101: 1ST - REAR EXT GREAT ROOM

SHEARWALL PROPERTIES:

WALL HEIGHT, H	<input type="text" value="10.0"/>	FT.	MAX WALL OPENING HT, H _c	<input type="text" value="4.5"/>	FT.		
WALL LENGTH, L	<input type="text" value="40.0"/>	FT.	QUALIFYING WALL LENGTH, L	<input type="text" value="28.0"/>	FT.	SHEARWALL ASSEMBLY	<input type="text" value="P1"/>

CAPACITY EVALUATION:

TOTAL SHEAR LOAD ON WALL	<input type="text" value="5700"/>	LBS	<	ALLOWABLE SHEARWALL CAPACITY	<input type="text" value="9402"/>	LBS
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SHEARWALL ASSEMBLY SPECIFICATION

P1 - 1-SIDE 7/16" OSB
FASTENED W/ 8D NAILS AT 6"O.C. PANEL EDGES & 12"O.C. PANEL FIELD - EDGES BLOCKED
ADEQUATE

OVERTURNING EVALUATION:

RESISTIVE DL	<input type="text" value="350"/>	PLF	OVERTURNING MOMENT	<input type="text" value="57.0"/>	K-FT	HOLD DOWN DESIGN LOAD	<input type="text" value="0"/>	LBS
DL AT ENDS OF WALL	<input type="text" value="400"/>	LBS	RESISTIVE MOMENT	<input type="text" value="136.5"/>	K-FT	HOLD DOWN CAPACITY	<input type="text" value="0"/>	LBS

HOLD-DOWN SPECIFICATION

NO HOLDOWN REQUIRED



SHEARWALL DESIGN SUMMARY

SHEARWALL 102: 1ST - FRONT EXT COVERED PATIO

SHEARWALL PROPERTIES:

WALL HEIGHT, H FT. MAX WALL OPENING HT, H_c FT.
WALL LENGTH, L FT. QUALIFYING WALL LENGTH, L FT. SHEARWALL ASSEMBLY

CAPACITY EVALUATION:

TOTAL SHEAR LOAD ON WALL LBS < ALLOWABLE SHEARWALL CAPACITY LBS

SHEARWALL ASSEMBLY SPECIFICATION

P3 - 1-SIDE 7/16" OSB
FASTENED W/ 8D NAILS AT 3"O.C. PANEL EDGES & 12"O.C. PANEL FIELD - EDGES BLOCKED
ADEQUATE

OVERTURNING EVALUATION:

RESISTIVE DL PLF OVERTURNING MOMENT K-FT HOLD DOWN DESIGN LOAD LBS
DL AT ENDS OF WALL LBS RESISTIVE MOMENT K-FT HOLDOWN CAPACITY LBS

HOLD-DOWN SPECIFICATION

NO HOLDOWN REQUIRED

SHEARWALL 103: 1ST - FRONT EXT BATH 1

SHEARWALL PROPERTIES:

WALL HEIGHT, H FT. MAX WALL OPENING HT, H_c FT.
WALL LENGTH, L FT. QUALIFYING WALL LENGTH, L FT. SHEARWALL ASSEMBLY

CAPACITY EVALUATION:

TOTAL SHEAR LOAD ON WALL LBS < ALLOWABLE SHEARWALL CAPACITY LBS

SHEARWALL ASSEMBLY SPECIFICATION

P1 - 1-SIDE 7/16" OSB
FASTENED W/ 8D NAILS AT 6"O.C. PANEL EDGES & 12"O.C. PANEL FIELD - EDGES BLOCKED
ADEQUATE

OVERTURNING EVALUATION:

RESISTIVE DL PLF OVERTURNING MOMENT K-FT HOLD DOWN DESIGN LOAD LBS
DL AT ENDS OF WALL LBS RESISTIVE MOMENT K-FT HOLDOWN CAPACITY LBS

HOLD-DOWN SPECIFICATION

NO HOLDOWN REQUIRED



SHEARWALL DESIGN SUMMARY

SHEARWALL 104: 1ST - FRONT EXT MUD

SHEARWALL PROPERTIES:

WALL HEIGHT, H	<input type="text" value="10.0"/>	FT.	MAX WALL OPENING HT, H _c	<input type="text" value="4.5"/>	FT.		
WALL LENGTH, L	<input type="text" value="8.7"/>	FT.	QUALIFYING WALL LENGTH, L	<input type="text" value="5.7"/>	FT.	SHEARWALL ASSEMBLY	<input type="text" value="P3"/>

CAPACITY EVALUATION:

TOTAL SHEAR LOAD ON WALL	<input type="text" value="1600"/>	LBS	<	ALLOWABLE SHEARWALL CAPACITY	<input type="text" value="3573"/>	LBS
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SHEARWALL ASSEMBLY SPECIFICATION

P3 - 1-SIDE 7/16" OSB
FASTENED W/ 8D NAILS AT 3"O.C. PANEL EDGES & 12"O.C. PANEL FIELD - EDGES BLOCKED
ADEQUATE

OVERTURNING EVALUATION:

RESISTIVE DL	<input type="text" value="243"/>	PLF	OVERTURNING MOMENT	<input type="text" value="16.0"/>	K-FT	HOLD DOWN DESIGN LOAD	<input type="text" value="1175"/>	LBS
DL AT ENDS OF WALL	<input type="text" value="400"/>	LBS	RESISTIVE MOMENT	<input type="text" value="5.8"/>	K-FT	HOLD DOWN CAPACITY	<input type="text" value="4935"/>	LBS

HOLD-DOWN SPECIFICATION

SIMPSON STDH14RJ HOLDOWN

SHEARWALL 105: 1ST - REAR EXT GARAGE

SHEARWALL PROPERTIES:

WALL HEIGHT, H	<input type="text" value="8.0"/>	FT.	MAX WALL OPENING HT, H _c	<input type="text" value="0.0"/>	FT.		
WALL LENGTH, L	<input type="text" value="19.5"/>	FT.	QUALIFYING WALL LENGTH, L	<input type="text" value="19.5"/>	FT.	SHEARWALL ASSEMBLY	<input type="text" value="P1"/>

CAPACITY EVALUATION:

TOTAL SHEAR LOAD ON WALL	<input type="text" value="4200"/>	LBS	<	ALLOWABLE SHEARWALL CAPACITY	<input type="text" value="6548"/>	LBS
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SHEARWALL ASSEMBLY SPECIFICATION

P1 - 1-SIDE 7/16" OSB
FASTENED W/ 8D NAILS AT 6"O.C. PANEL EDGES & 12"O.C. PANEL FIELD - EDGES BLOCKED
ADEQUATE

OVERTURNING EVALUATION:

RESISTIVE DL	<input type="text" value="287"/>	PLF	OVERTURNING MOMENT	<input type="text" value="33.6"/>	K-FT	HOLD DOWN DESIGN LOAD	<input type="text" value="0"/>	LBS
DL AT ENDS OF WALL	<input type="text" value="1000"/>	LBS	RESISTIVE MOMENT	<input type="text" value="34.1"/>	K-FT	HOLD DOWN CAPACITY	<input type="text" value="0"/>	LBS

HOLD-DOWN SPECIFICATION

NO HOLDOWN REQUIRED



SHEARWALL DESIGN SUMMARY

SHEARWALL XXX: - NOT USED

SHEARWALL PROPERTIES:

WALL HEIGHT, H FT. MAX WALL OPENING HT, H_c FT.
WALL LENGTH, L FT. QUALIFYING WALL LENGTH, L FT. SHEARWALL ASSEMBLY

CAPACITY EVALUATION:

TOTAL SHEAR LOAD ON WALL LBS ALLOWABLE SHEARWALL CAPACITY LBS
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SHEARWALL ASSEMBLY SPECIFICATION

PO - 1-SIDE 7/16" OSB
FASTENED W/ 8D NAILS AT 6"O.C. PANEL EDGES & 12"O.C. PANEL FIELD - UNBLOCKED
#DIV/0!

OVERTURNING EVALUATION:

RESISTIVE DL PLF OVERTURNING MOMENT K-FT HOLD DOWN DESIGN LOAD LBS
DL AT ENDS OF WALL LBS RESISTIVE MOMENT K-FT HOLDOWN CAPACITY LBS

HOLD-DOWN SPECIFICATION

NO HOLDOWN REQUIRED

SHEARWALL 106: 1ST - SIDE EXT GARAGE

SHEARWALL PROPERTIES:

WALL HEIGHT, H FT. MAX WALL OPENING HT, H_c FT.
WALL LENGTH, L FT. QUALIFYING WALL LENGTH, L FT. SHEARWALL ASSEMBLY

CAPACITY EVALUATION:

TOTAL SHEAR LOAD ON WALL LBS ALLOWABLE SHEARWALL CAPACITY LBS
<

SHEARWALL ASSEMBLY SPECIFICATION

P1 - 1-SIDE 7/16" OSB
FASTENED W/ 8D NAILS AT 6"O.C. PANEL EDGES & 12"O.C. PANEL FIELD - EDGES BLOCKED
ADEQUATE

OVERTURNING EVALUATION:

RESISTIVE DL PLF OVERTURNING MOMENT K-FT HOLD DOWN DESIGN LOAD LBS
DL AT ENDS OF WALL LBS RESISTIVE MOMENT K-FT HOLDOWN CAPACITY LBS

HOLD-DOWN SPECIFICATION

SIMPSON STHD14RJ HOLDOWN



SHEARWALL DESIGN SUMMARY

SHEARWALL 107: 1ST - SIDE EXT DINING

SHEARWALL PROPERTIES:

WALL HEIGHT, H FT. MAX WALL OPENING HT, H_c FT.
WALL LENGTH, L FT. QUALIFYING WALL LENGTH, L FT. SHEARWALL ASSEMBLY

CAPACITY EVALUATION:

TOTAL SHEAR LOAD ON WALL LBS < ALLOWABLE SHEARWALL CAPACITY LBS

SHEARWALL ASSEMBLY SPECIFICATION

P3 - 1-SIDE 7/16" OSB
FASTENED W/ 8D NAILS AT 3"O.C. PANEL EDGES & 12"O.C. PANEL FIELD - EDGES BLOCKED
ADEQUATE

OVERTURNING EVALUATION:

RESISTIVE DL PLF OVERTURNING MOMENT K-FT HOLD DOWN DESIGN LOAD LBS
DL AT ENDS OF WALL LBS RESISTIVE MOMENT K-FT HOLDOWN CAPACITY LBS

HOLD-DOWN SPECIFICATION

SIMPSON STDH14RJ HOLDOWN

SHEARWALL 108: 1ST - SIDE INT GARAGE

SHEARWALL PROPERTIES:

WALL HEIGHT, H FT. MAX WALL OPENING HT, H_c FT.
WALL LENGTH, L FT. QUALIFYING WALL LENGTH, L FT. SHEARWALL ASSEMBLY

CAPACITY EVALUATION:

TOTAL SHEAR LOAD ON WALL LBS < ALLOWABLE SHEARWALL CAPACITY LBS

SHEARWALL ASSEMBLY SPECIFICATION

P3 - 1-SIDE 7/16" OSB
FASTENED W/ 8D NAILS AT 3"O.C. PANEL EDGES & 12"O.C. PANEL FIELD - EDGES BLOCKED
ADEQUATE

OVERTURNING EVALUATION:

RESISTIVE DL PLF OVERTURNING MOMENT K-FT HOLD DOWN DESIGN LOAD LBS
DL AT ENDS OF WALL LBS RESISTIVE MOMENT K-FT HOLDOWN CAPACITY LBS

HOLD-DOWN SPECIFICATION

SIMPSON STDH14RJ HOLDOWN



SHEARWALL DESIGN SUMMARY

SHEARWALL 109: 1ST - SIDE EXT STAIRS / GREAT ROOM

SHEARWALL PROPERTIES:

WALL HEIGHT, H FT. MAX WALL OPENING HT, H_c FT.
WALL LENGTH, L FT. QUALIFYING WALL LENGTH, L FT. SHEARWALL ASSEMBLY

CAPACITY EVALUATION:

TOTAL SHEAR LOAD ON WALL LBS < ALLOWABLE SHEARWALL CAPACITY LBS

SHEARWALL ASSEMBLY SPECIFICATION

P3 - 1-SIDE 7/16" OSB
FASTENED W/ 8D NAILS AT 3"O.C. PANEL EDGES & 12"O.C. PANEL FIELD - EDGES BLOCKED
ADEQUATE

OVERTURNING EVALUATION:

RESISTIVE DL PLF OVERTURNING MOMENT K-FT HOLD DOWN DESIGN LOAD LBS
DL AT ENDS OF WALL LBS RESISTIVE MOMENT K-FT HOLDOWN CAPACITY LBS

HOLD-DOWN SPECIFICATION

SIMPSON STD14RJ HOLDOWN

SHEARWALL 110: 1ST - SIDE EXT COVERED PATIO

SHEARWALL PROPERTIES:

WALL HEIGHT, H FT. MAX WALL OPENING HT, H_c FT.
WALL LENGTH, L FT. QUALIFYING WALL LENGTH, L FT. SHEARWALL ASSEMBLY

CAPACITY EVALUATION:

TOTAL SHEAR LOAD ON WALL LBS < ALLOWABLE SHEARWALL CAPACITY LBS

SHEARWALL ASSEMBLY SPECIFICATION

P3 - 1-SIDE 7/16" OSB
FASTENED W/ 8D NAILS AT 3"O.C. PANEL EDGES & 12"O.C. PANEL FIELD - EDGES BLOCKED
ADEQUATE

OVERTURNING EVALUATION:

RESISTIVE DL PLF OVERTURNING MOMENT K-FT HOLD DOWN DESIGN LOAD LBS
DL AT ENDS OF WALL LBS RESISTIVE MOMENT K-FT HOLDOWN CAPACITY LBS

HOLD-DOWN SPECIFICATION

NO HOLDOWN REQUIRED

JAYMARC HOMES
SPRING RESIDENCE

MERCER ISLAND, WA

SHEAR WALL CALCULATIONS - SEISMIC

REVIEWED BY: RJZ

NOVEMBER 1, 2022

PARAMETERS:

SINGLE FAMILY HOME

DESIGN WIND SPEED: 100 MPH

WIND EXPOSURE CATEGORY: C

SEISMIC DESIGN CATEGORY: D

CODE & DESIGN STANDARD: 2018 IBC CH. 1609, ASCE 7-16 CH. 26-30



MULHERN+KULP
RESIDENTIAL STRUCTURAL ENGINEERING

SEISMIC CALCULATION - ASCE 7-16

SEISMIC DESIGN CATEGORY:

USER INPUTS:

SITE CLASS	D
SPECTRAL RESPONSE ACCELERATION 0.2 SEC, S_s	1.440
SPECTRAL RESPONSE ACCELERATION 1.0 SEC, S₁	0.500
OCCUPANCY CATEGORY	II

VARIABLES:

SITE COEFFICIENT, F _A	1.20
SITE COEFFICIENT, F _V	1.80

CALCULATED VALUES:

MAXIMUM SPECTRAL RESPONSE ACCELERATION, S_{M8}	1.728
MAXIMUM SPECTRAL RESPONSE ACCELERATION, S_{M1}	0.900
DESIGN SPECTRAL RESPONSE ACCELERATION, S_{D8}	1.152
DESIGN SPECTRAL RESPONSE ACCELERATION, S_{D1}	0.600
SEISMIC DESIGN CATEGORY (SHORT TERM)	D
SEISMIC DESIGN CATEGORY (1.0 SECOND TERM)	D

BUILDING PERIOD DETERMINATION:

USER INPUTS:

BUILDING PERIOD COEFFICIENT, C_T	0.020
LONG-PERIOD TRANS PERIOD, T_L (SEC)	6
HT. ABV BASE TO HIGHEST LEVEL, h_N	21

CALCULATED VALUES:

APPROXIMATE FUNDAMENTAL PERIOD, T_A	0.193
T₀	0.104
T_S	0.521
SPECTRAL RESPONSE ACC., S_A (g)	1.152

SITE CLASS ASSUMPTION

YES PER ASCE 7-16 SECTION 11.4.3 THE SITE CLASS MAY BE ASSUMED TO BE D

EQUIVALENT LATERAL FORCE PROCEDURE

DEAD LOAD CALCULATION:

LEVEL	STORY HT. (FT.)	AREA (FT ²)	DEAD LOAD (PSF)	DL OF EXT WALL TRIB. TO LEVEL (KIPS)	TOTAL LEVEL DL
1	11.5	3700	15	16.2	72 K
2	9.1	2846	17	7.5	56 K
3	0.0	0	0	0.0	0 K
4	0.0	0	0	0.0	0 K
5	0.0	0	0	0.0	0 K
6	0.0	0	0	0.0	0 K
7	0.0	0	0	0.0	0 K
8	0.0	0	0	0.0	0 K
9	0.0	0	0	0.0	0 K
10	0.0	0	0	0.0	0 K
11	0.0	0	0	0.0	0 K
12	0.0	0	0	0.0	0 K
13	0.0	0	0	0.0	0 K
14	0.0	0	0	0.0	0 K
15	0.0	0	0	0.0	0 K
16	0.0	0	0	0.0	0 K
17	0.0	0	0	0.0	0 K
18	0.0	0	0	0.0	0 K
19	0.0	0	0	0.0	0 K
20	0.0	0	0	0.0	0 K

TOTAL DEAD LOAD OF STRUCTURE 128 KIPS

SEISMIC RESPONSE COEFFICIENT:

	TRANSVERSE	LONGITUDINAL
RESPONSE MODIFICATION FACTOR, R	6.5	6.5
OCCUPANCY IMPORTANCE FACTOR, I_e	1.00	1.00
SEISMIC RESPONSE COEFFICIENT, C_s	0.177	0.177

BASE SHEARS:

ULTIMATE LOADS

x 0.7 =

ALLOWABLE LOADS

TRANSVERSE	LONGITUDINAL	TRANSVERSE	LONGITUDINAL
23 K	23 K	15.8 K	15.8 K

STORY SHEAR CALCULATION:

DISTRIBUTION EXPONENT, **γ** 1.00

ULTIMATE LOADS

x 0.7 =

ALLOWABLE LOADS

LEVEL	VERT. DIST. FACTOR, C_{vk}	TRANSVERSE		LONGITUDINAL		TRANSVERSE		LONGITUDINAL	
		STORY SHEAR, F _x	STORY SHEAR, F _y	STORY SHEAR, F _x	STORY SHEAR, F _y	STORY SHEAR, F _x	STORY SHEAR, F _y	STORY SHEAR, F _x	STORY SHEAR, F _y
1	0.417	9.4	9.4	6.6	15.8	6.6	15.8		
2	0.583	13.2	13.2	9.2	9.2	9.2	9.2		
3	0.000	0.0	0.0	0.0	0.0	0.0	0.0		
4	0.000	0.0	0.0	0.0	0.0	0.0	0.0		
5	0.00	0.0	0.0	0.0	0.0	0.0	0.0		
6	0.00	0.0	0.0	0.0	0.0	0.0	0.0		
7	0.00	0.0	0.0	0.0	0.0	0.0	0.0		
8	0.00	0.0	0.0	0.0	0.0	0.0	0.0		
9	0.00	0.0	0.0	0.0	0.0	0.0	0.0		
10	0.00	0.0	0.0	0.0	0.0	0.0	0.0		
11	0.00	0.0	0.0	0.0	0.0	0.0	0.0		
12	0.00	0.0	0.0	0.0	0.0	0.0	0.0		
13	0.00	0.0	0.0	0.0	0.0	0.0	0.0		
14	0.00	0.0	0.0	0.0	0.0	0.0	0.0		
15	0.00	0.0	0.0	0.0	0.0	0.0	0.0		
16	0.00	0.0	0.0	0.0	0.0	0.0	0.0		
17	0.00	0.0	0.0	0.0	0.0	0.0	0.0		
18	0.00	0.0	0.0	0.0	0.0	0.0	0.0		
19	0.00	0.0	0.0	0.0	0.0	0.0	0.0		
20	0.00	0.0	0.0	0.0	0.0	0.0	0.0		

plan name: ---
 marketing name: ---
 plan number: ---
 mark sys. number: ---

Conditions not specifically represented graphically or in writing or which conflict with the current International Residential Code (IRC), or those of the local municipality then the current standards and requirements of each respectively shall govern.

The drawings in this set are instruments of service and shall remain the property of JayMarc Homes, LLC.

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11.22.22
 Submittal Date

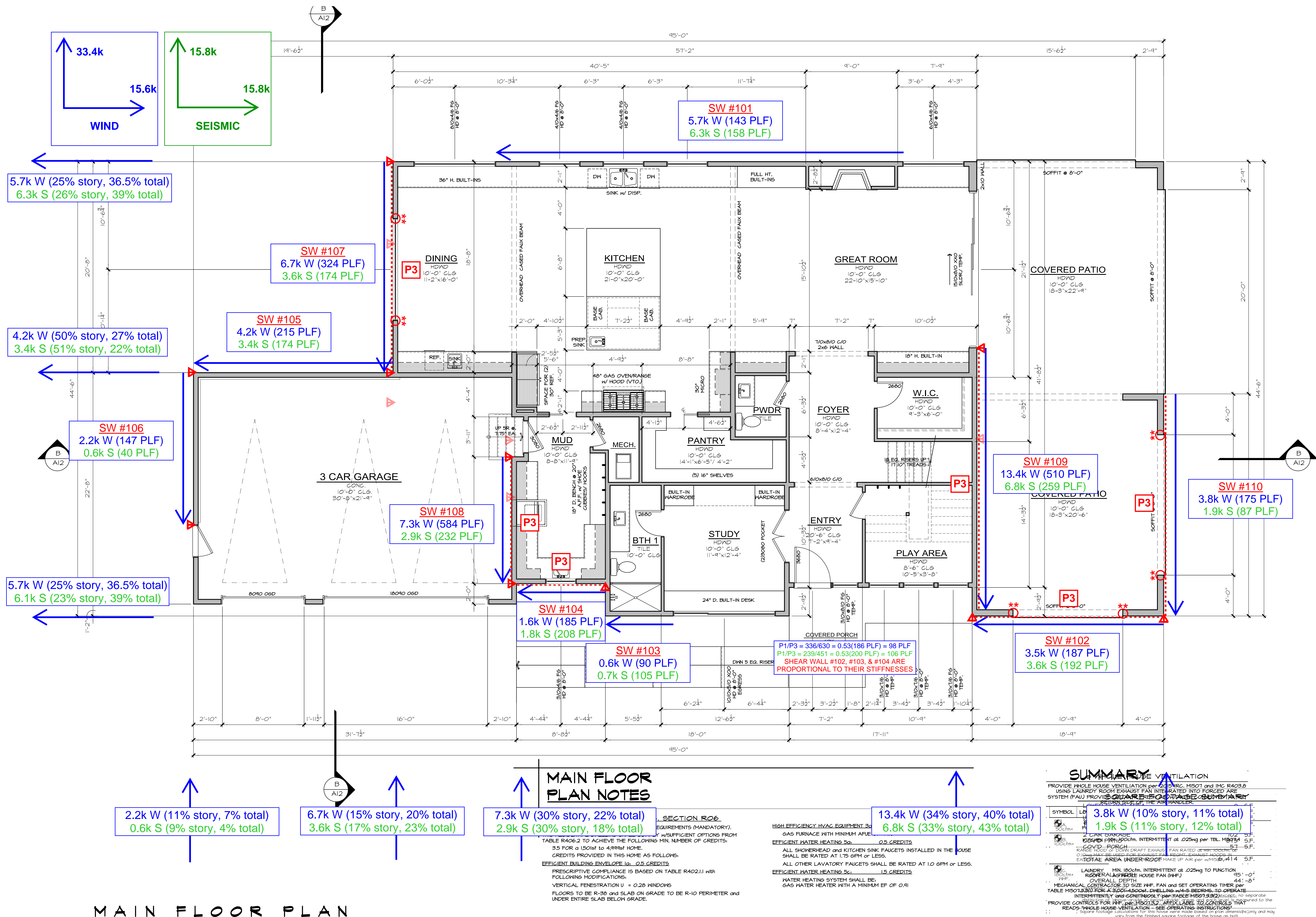
Sheet Title/Description
 JAYMARC HOMES
 Design Firm

R.K.N.
 Drawn by:

Checked by:

Primary Scale

A5
 of .



MAIN FLOOR PLAN NOTES

SECTION R06
 EQUIPMENTS (MANDATORY).
 REQUIREMENTS (MANDATORY).
 W/SUFFICIENT OPTIONS FROM
 TABLE R406.2 TO ACHIEVE THE FOLLOWING MIN. NUMBER OF CREDITS:
 3.5 FOR A 1501sf to 4,999sf HOME.
 CREDITS PROVIDED IN THIS HOME AS FOLLOWS:
 EFFICIENT BUILDING ENVELOPE (a) .05 CREDITS
 PRESCRIPTIVE COMPLIANCE IS BASED ON TABLE R402.1.1 WITH
 FOLLOWING MODIFICATIONS:
 VERTICAL FENESTRATION U = 0.29 WINDOWS
 FLOORS TO BE R-39 and SLAB ON GRADE TO BE R-10 PERIMETER and
 UNDER ENTIRE SLAB BELOW GRADE.

HIGH EFFICIENCY HVAC EQUIPMENT 3c
 GAS FURNACE WITH MINIMUM AFUE
 EFFICIENT WATER HEATING 5a: .05 CREDITS
 ALL SHOWERHEAD AND KITCHEN SINK FAUCETS INSTALLED IN THE HOUSE
 SHALL BE RATED AT 1.75 GPM or LESS.
 ALL OTHER LAVATORY FAUCETS SHALL BE RATED AT 1.0 GPM or LESS.
 EFFICIENT WATER HEATING 5a: 1.5 CREDITS
 WATER HEATING SYSTEM SHALL BE:
 GAS WATER HEATER WITH A MINIMUM EF OF 0.91

SUMMARY

SYMBOL	DESCRIPTION	AMOUNT	PERCENTAGE
SW #101	3.8k W (10% story, 11% total)	3.8k W	10% story, 11% total
SW #102	1.9k S (11% story, 12% total)	1.9k S	11% story, 12% total
SW #103	13.4k W (34% story, 40% total)	13.4k W	34% story, 40% total
SW #104	6.8k S (33% story, 43% total)	6.8k S	33% story, 43% total

Issue	Issue Date	By	Description

Spring Residence
 4740 W. Mercer Way
 Mercer Island, WA.
 Job Number: Spring

plan name:	-
marketing name:	-
plan number:	-
mark sys. number:	-

Conditions not specifically represented graphically or in writing or which conflict with the current International Residential Code (IRC), or those of the local municipality and then the current standards and requirements of each respectively shall govern.

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11.22.22
 Submittal Date

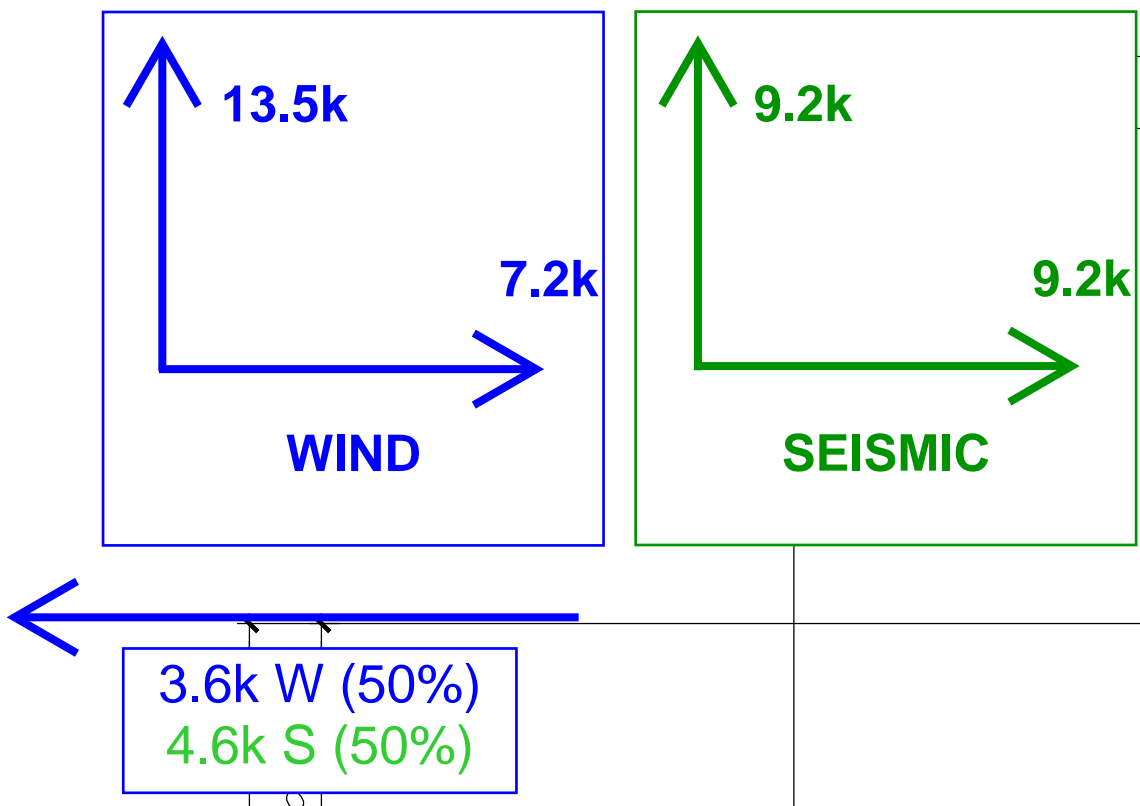
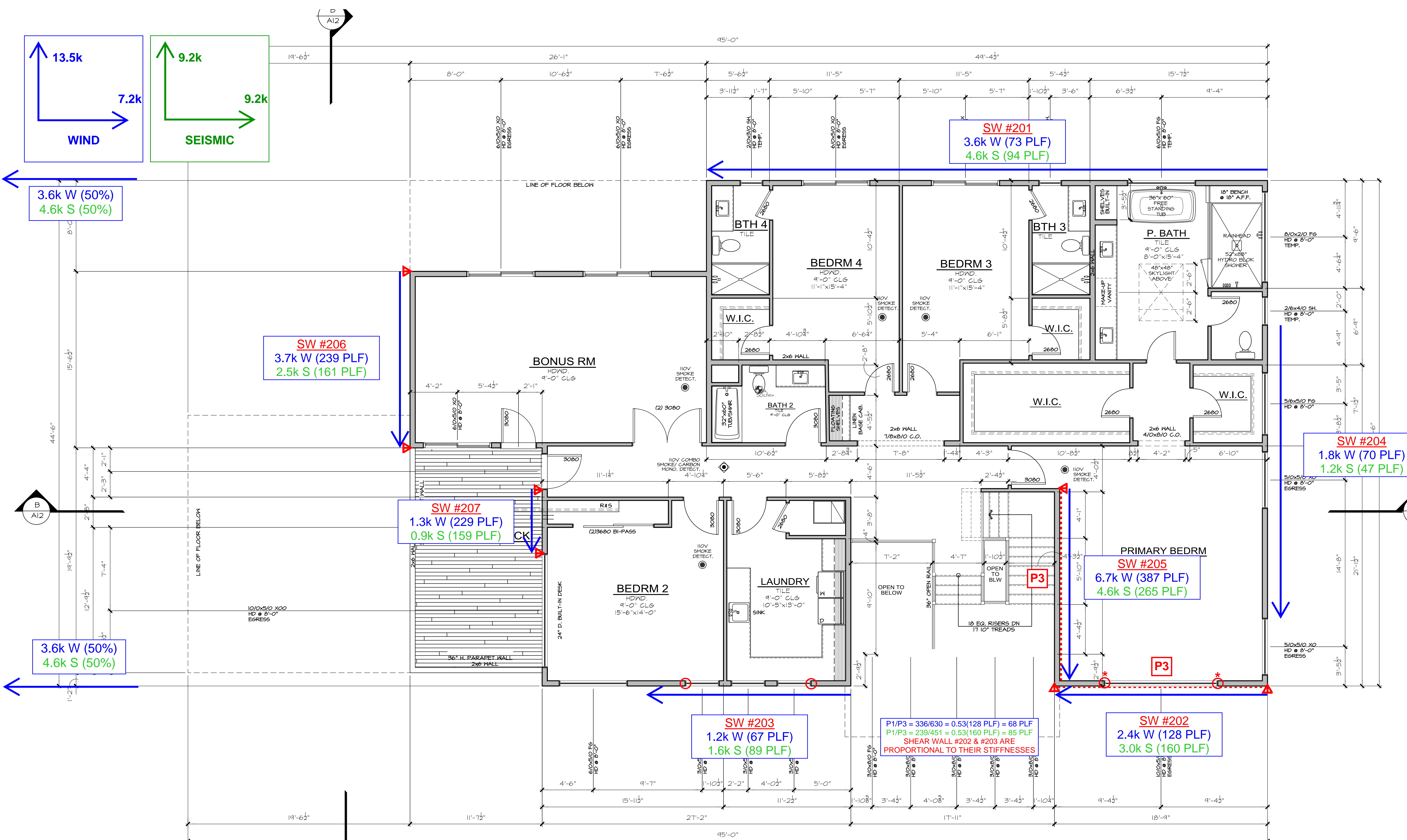
Sheet Title/Description
 JAYMARC HOMES
 Design Firm

R.K.N.
 Drawn by:

Checked by:

Primary Scale

A7
 of .



3.6k W (50%)
 4.6k S (50%)

SW #206
 3.7k W (239 PLF)
 2.5k S (161 PLF)

SW #207
 1.3k W (229 PLF)
 0.9k S (159 PLF)

SW #203
 1.2k W (67 PLF)
 1.6k S (89 PLF)

P1/P3 = 336/630 = 0.53(128 PLF) = 68 PLF
 P1/P3 = 239/451 = 0.53(160 PLF) = 85 PLF
 SHEAR WALL #202 & #203 ARE
 PROPORTIONAL TO THEIR STIFFNESSES

SW #202
 2.4k W (128 PLF)
 3.0k S (160 PLF)

SW #205
 6.7k W (387 PLF)
 4.6k S (265 PLF)

SW #204
 1.8k W (70 PLF)
 1.2k S (47 PLF)

UPPER FLOOR PLAN NOTES:

PLAN SPECIFIC 2015 NSEC. SECT. R406.2 ADDITIONAL ENERGY EFFICIENCY REQUIREMENT THIS RESIDENTIAL DWELLING SHALL COMPLY WITH SUFFICIENT TABLE R406.2 TO ACHIEVE THE FOLLOWING MIN. NUMBER CREDITS PROVIDED IN THIS HOME AS FOLLOWS:
 EFFICIENT BUILDING ENVELOPE 1a - 0.5 CREDITS
 PRESCRIPTIVE COMPLIANCE IS BASED ON TABLE R402.1.1 WITH FOLLOWING MODIFICATIONS:
 VERTICAL PENETRATION U = 0.28 WINDOWS
 FLOORS TO BE R-38 and SLAB ON GRADE TO BE R-10 PERIMETER and UNDER ENTIRE SLAB BELOW GRADE.

6.2 ADDITIONAL ENERGY EFFICIENCY REQUIREMENTS (MANDATORY).
 EFFICIENT HVAC EQUIPMENT 3a - 1.0 CREDITS
 GAS FURNACE WITH MINIMUM AFUE OF 94%
 EFFICIENT WATER HEATING 5a - 0.5 CREDITS
 ALL SHOWERHEAD and KITCHEN SINK FAUCETS INSTALLED IN THE HOUSE SHALL BE RATED AT 1.75 GPM or LESS.
 ALL OTHER LAVATORY FAUCETS SHALL BE RATED AT 1.0 GPM or LESS.
 EFFICIENT WATER HEATING 5c - 1.5 CREDITS
 WATER HEATING SYSTEM SHALL BE:
 GAS WATER HEATER WITH A MINIMUM EF OF 0.91

WHOLE HOUSE VENTILATION

PROVIDE WHOLE HOUSE VENTILATION per 2015 IRC, M501 and IMC R403.8 USING LAUNDRY ROOM EXHAUST FAN INTEGRATED INTO FORCED AIR SYSTEM (FAU) PROVIDE OUTDOOR FRESH AIR W/DUCTS CONNECTED TO THE RETURN SIDE OF THE AIR HANDLER.

SYMBOL	LOCATION	MIN. FAN REQUIREMENTS (ALL FANS VENT TO OUTSIDE)
	BATH #1 POWDER ROOM	Min. 50cfm, INTERMITTENT at .025kg per TABLE M501.4
	KITCHEN	Min. 100cfm, INTERMITTENT at .025kg per TBL. M501.4
	LAUNDRY ROOM	MIN. 180cfm, INTERMITTENT at .025kg to FUNCTION AS WHOLE HOUSE FAN (WHF)

MECHANICAL CONTRACTOR TO SIZE WHF, FAN and SET OPERATING TIMER per TABLE M501.3(3) FOR A 3001-4500H. DWELLING w/4-5 BEDRMS. TO OPERATE INTERMITTENTLY and CONTINUOUSLY per TABLE M501.3(2)
 PROVIDE CONTROLS FOR WHF, per M501.3.2 AFFIX LABEL TO CONTROLS THAT READS "WHOLE HOUSE VENTILATION - SEE OPERATING INSTRUCTIONS"

SUMMARY

SQUARE FOOTAGE SUMMARY	
LOWER FLOOR AREA	702 S.F.
MAIN FLOOR AREA	915 S.F.
UPPER FLOOR AREA	57 S.F.
TOTAL CONDITIONED AREA	1674 S.F.
2 CAR GARAGE	702 S.F.
COVID PATIO	57 S.F.
COVID PORCH	57 S.F.
TOTAL AREA UNDER ROOF	6,414 S.F.
OVERALL WIDTH	45'-0"
OVERALL DEPTH	44'-8"

Updated: 12.09.20
 Method for Calculating Square Footage - ANSI Z765-2019 excludes no separate distinction of above-grade or below-grade areas and each level is measured to the outside of studs not the exterior finished surface.
 Square footage calculations for this house were made based on plan dimensions only and may vary from square footage shown on other house drawings.

UPPER FLOOR PLAN

1/4" = 1'-0"

Sheet Title/Description



SHEARWALL DESIGN SUMMARY

SHEARWALL 201: 2ND - REAR EXT BED 4 TO PRIMARY BATH

SHEARWALL PROPERTIES:

WALL HEIGHT, H FT. MAX WALL OPENING HT, H_c FT.
WALL LENGTH, L FT. QUALIFYING WALL LENGTH, L FT. SHEARWALL ASSEMBLY

CAPACITY EVALUATION:

TOTAL SHEAR LOAD ON WALL LBS < ALLOWABLE SHEARWALL CAPACITY LBS

SHEARWALL ASSEMBLY SPECIFICATION

P1 - 1-SIDE 7/16" OSB
FASTENED W/ 8D NAILS AT 6"O.C. PANEL EDGES & 12"O.C. PANEL FIELD - EDGES BLOCKED
ADEQUATE

OVERTURNING EVALUATION:

RESISTIVE DL PLF OVERTURNING MOMENT K-FT HOLD DOWN DESIGN LOAD LBS
DL AT ENDS OF WALL LBS RESISTIVE MOMENT K-FT HOLDOWN CAPACITY LBS

HOLD-DOWN SPECIFICATION

NO HOLDOWN REQUIRED

SHEARWALL 202: 2ND - FRONT EXT PRIMARY BEDROOM

SHEARWALL PROPERTIES:

WALL HEIGHT, H FT. MAX WALL OPENING HT, H_c FT.
WALL LENGTH, L FT. QUALIFYING WALL LENGTH, L FT. SHEARWALL ASSEMBLY

CAPACITY EVALUATION:

TOTAL SHEAR LOAD ON WALL LBS < ALLOWABLE SHEARWALL CAPACITY LBS

SHEARWALL ASSEMBLY SPECIFICATION

P3 - 1-SIDE 7/16" OSB
FASTENED W/ 8D NAILS AT 3"O.C. PANEL EDGES & 12"O.C. PANEL FIELD - EDGES BLOCKED
ADEQUATE

OVERTURNING EVALUATION:

RESISTIVE DL PLF OVERTURNING MOMENT K-FT HOLD DOWN DESIGN LOAD LBS
DL AT ENDS OF WALL LBS RESISTIVE MOMENT K-FT HOLDOWN CAPACITY LBS

HOLD-DOWN SPECIFICATION

SIMPSON GS16 STRAP TIE (14" END LENGTH)



SHEARWALL DESIGN SUMMARY

SHEARWALL 203: 2ND - FRONT EXT BED 2 / LAUNDRY

SHEARWALL PROPERTIES:

WALL HEIGHT, H FT. MAX WALL OPENING HT, H_c FT.
WALL LENGTH, L FT. QUALIFYING WALL LENGTH, L FT. SHEARWALL ASSEMBLY

CAPACITY EVALUATION:

TOTAL SHEAR LOAD ON WALL LBS < ALLOWABLE SHEARWALL CAPACITY LBS

SHEARWALL ASSEMBLY SPECIFICATION

P1 - 1-SIDE 7/16" OSB
FASTENED W/ 8D NAILS AT 6"O.C. PANEL EDGES & 12"O.C. PANEL FIELD - EDGES BLOCKED
ADEQUATE

OVERTURNING EVALUATION:

RESISTIVE DL PLF OVERTURNING MOMENT K-FT HOLD DOWN DESIGN LOAD LBS
DL AT ENDS OF WALL LBS RESISTIVE MOMENT K-FT HOLDOWN CAPACITY LBS

HOLD-DOWN SPECIFICATION

NO HOLDOWN REQUIRED

SHEARWALL 204: 2ND - SIDE EXT PRIMARY BEDROOM / BATH

SHEARWALL PROPERTIES:

WALL HEIGHT, H FT. MAX WALL OPENING HT, H_c FT.
WALL LENGTH, L FT. QUALIFYING WALL LENGTH, L FT. SHEARWALL ASSEMBLY

CAPACITY EVALUATION:

TOTAL SHEAR LOAD ON WALL LBS < ALLOWABLE SHEARWALL CAPACITY LBS

SHEARWALL ASSEMBLY SPECIFICATION

P1 - 1-SIDE 7/16" OSB
FASTENED W/ 8D NAILS AT 6"O.C. PANEL EDGES & 12"O.C. PANEL FIELD - EDGES BLOCKED
ADEQUATE

OVERTURNING EVALUATION:

RESISTIVE DL PLF OVERTURNING MOMENT K-FT HOLD DOWN DESIGN LOAD LBS
DL AT ENDS OF WALL LBS RESISTIVE MOMENT K-FT HOLDOWN CAPACITY LBS

HOLD-DOWN SPECIFICATION

NO HOLDOWN REQUIRED



SHEARWALL DESIGN SUMMARY

SHEARWALL 205: 2ND - SIDE INT PRIMARY BEDROOM

SHEARWALL PROPERTIES:

WALL HEIGHT, H FT. MAX WALL OPENING HT, H_c FT.
WALL LENGTH, L FT. QUALIFYING WALL LENGTH, L FT. SHEARWALL ASSEMBLY

CAPACITY EVALUATION:

TOTAL SHEAR LOAD ON WALL LBS < ALLOWABLE SHEARWALL CAPACITY LBS

SHEARWALL ASSEMBLY SPECIFICATION

P3 - 1-SIDE 7/16" OSB
FASTENED W/ 8D NAILS AT 3"O.C. PANEL EDGES & 12"O.C. PANEL FIELD - EDGES BLOCKED
ADEQUATE

OVERTURNING EVALUATION:

RESISTIVE DL PLF OVERTURNING MOMENT K-FT HOLD DOWN DESIGN LOAD LBS
DL AT ENDS OF WALL LBS RESISTIVE MOMENT K-FT HOLDOWN CAPACITY LBS

HOLD-DOWN SPECIFICATION

SIMPSON MSTC66 STRAP TIE (20" END LENGTH)

SHEARWALL 206: 2ND - SIDE EXT BONUS

SHEARWALL PROPERTIES:

WALL HEIGHT, H FT. MAX WALL OPENING HT, H_c FT.
WALL LENGTH, L FT. QUALIFYING WALL LENGTH, L FT. SHEARWALL ASSEMBLY

CAPACITY EVALUATION:

TOTAL SHEAR LOAD ON WALL LBS < ALLOWABLE SHEARWALL CAPACITY LBS

SHEARWALL ASSEMBLY SPECIFICATION

P1 - 1-SIDE 7/16" OSB
FASTENED W/ 8D NAILS AT 6"O.C. PANEL EDGES & 12"O.C. PANEL FIELD - EDGES BLOCKED
ADEQUATE

OVERTURNING EVALUATION:

RESISTIVE DL PLF OVERTURNING MOMENT K-FT HOLD DOWN DESIGN LOAD LBS
DL AT ENDS OF WALL LBS RESISTIVE MOMENT K-FT HOLDOWN CAPACITY LBS

HOLD-DOWN SPECIFICATION

SIMPSON GS16 STRAP TIE (14" END LENGTH)



SHEARWALL DESIGN SUMMARY

SHEARWALL 207: 2ND - SIDE EXT BED 2

SHEARWALL PROPERTIES:

WALL HEIGHT, H	<input type="text" value="9.0"/>	FT.	MAX WALL OPENING HT, H _c	<input type="text" value="0.0"/>	FT.		
WALL LENGTH, L	<input type="text" value="5.7"/>	FT.	QUALIFYING WALL LENGTH, L	<input type="text" value="5.7"/>	FT.	SHEARWALL ASSEMBLY	<input type="text" value="P1"/>

CAPACITY EVALUATION:

TOTAL SHEAR LOAD ON WALL	<input type="text" value="900"/>	LBS	<	ALLOWABLE SHEARWALL CAPACITY	<input type="text" value="1360"/>	LBS
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SHEARWALL ASSEMBLY SPECIFICATION

P1 - 1-SIDE 7/16" OSB
FASTENED W/ 8D NAILS AT 6"O.C. PANEL EDGES & 12"O.C. PANEL FIELD - EDGES BLOCKED
ADEQUATE

OVERTURNING EVALUATION:

RESISTIVE DL	<input type="text" value="150"/>	PLF	OVERTURNING MOMENT	<input type="text" value="8.1"/>	K-FT	HOLD DOWN DESIGN LOAD	<input type="text" value="1048"/>	LBS
DL AT ENDS OF WALL	<input type="text" value="400"/>	LBS	RESISTIVE MOMENT	<input type="text" value="2.2"/>	K-FT	HOLD DOWN CAPACITY	<input type="text" value="1705"/>	LBS

HOLD-DOWN SPECIFICATION

SIMPSON CS16 STRAP TIE (14" END LENGTH)

SHEARWALL 101: 1ST - REAR EXT GREAT ROOM

SHEARWALL PROPERTIES:

WALL HEIGHT, H	<input type="text" value="10.0"/>	FT.	MAX WALL OPENING HT, H _c	<input type="text" value="4.5"/>	FT.		
WALL LENGTH, L	<input type="text" value="40.0"/>	FT.	QUALIFYING WALL LENGTH, L	<input type="text" value="28.0"/>	FT.	SHEARWALL ASSEMBLY	<input type="text" value="P1"/>

CAPACITY EVALUATION:

TOTAL SHEAR LOAD ON WALL	<input type="text" value="6300"/>	LBS	<	ALLOWABLE SHEARWALL CAPACITY	<input type="text" value="6716"/>	LBS
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SHEARWALL ASSEMBLY SPECIFICATION

P1 - 1-SIDE 7/16" OSB
FASTENED W/ 8D NAILS AT 6"O.C. PANEL EDGES & 12"O.C. PANEL FIELD - EDGES BLOCKED
ADEQUATE

OVERTURNING EVALUATION:

RESISTIVE DL	<input type="text" value="350"/>	PLF	OVERTURNING MOMENT	<input type="text" value="63.0"/>	K-FT	HOLD DOWN DESIGN LOAD	<input type="text" value="0"/>	LBS
DL AT ENDS OF WALL	<input type="text" value="400"/>	LBS	RESISTIVE MOMENT	<input type="text" value="136.5"/>	K-FT	HOLD DOWN CAPACITY	<input type="text" value="0"/>	LBS

HOLD-DOWN SPECIFICATION

NO HOLDOWN REQUIRED



SHEARWALL DESIGN SUMMARY

SHEARWALL 102: 1ST - FRONT EXT COVERED PATIO

SHEARWALL PROPERTIES:

WALL HEIGHT, H FT. MAX WALL OPENING HT, H_c FT.
WALL LENGTH, L FT. QUALIFYING WALL LENGTH, L FT. SHEARWALL ASSEMBLY

CAPACITY EVALUATION:

TOTAL SHEAR LOAD ON WALL LBS < ALLOWABLE SHEARWALL CAPACITY LBS

SHEARWALL ASSEMBLY SPECIFICATION

P3 - 1-SIDE 7/16" OSB
FASTENED W/ 8D NAILS AT 3"O.C. PANEL EDGES & 12"O.C. PANEL FIELD - EDGES BLOCKED
ADEQUATE

OVERTURNING EVALUATION:

RESISTIVE DL PLF OVERTURNING MOMENT K-FT HOLD DOWN DESIGN LOAD LBS
DL AT ENDS OF WALL LBS RESISTIVE MOMENT K-FT HOLDOWN CAPACITY LBS

HOLD-DOWN SPECIFICATION

NO HOLDOWN REQUIRED

SHEARWALL 103: 1ST - FRONT EXT BATH 1

SHEARWALL PROPERTIES:

WALL HEIGHT, H FT. MAX WALL OPENING HT, H_c FT.
WALL LENGTH, L FT. QUALIFYING WALL LENGTH, L FT. SHEARWALL ASSEMBLY

CAPACITY EVALUATION:

TOTAL SHEAR LOAD ON WALL LBS < ALLOWABLE SHEARWALL CAPACITY LBS

SHEARWALL ASSEMBLY SPECIFICATION

P1 - 1-SIDE 7/16" OSB
FASTENED W/ 8D NAILS AT 6"O.C. PANEL EDGES & 12"O.C. PANEL FIELD - EDGES BLOCKED
ADEQUATE

OVERTURNING EVALUATION:

RESISTIVE DL PLF OVERTURNING MOMENT K-FT HOLD DOWN DESIGN LOAD LBS
DL AT ENDS OF WALL LBS RESISTIVE MOMENT K-FT HOLDOWN CAPACITY LBS

HOLD-DOWN SPECIFICATION

NO HOLDOWN REQUIRED



SHEARWALL DESIGN SUMMARY

SHEARWALL 104: 1ST - FRONT EXT MUD

SHEARWALL PROPERTIES:

WALL HEIGHT, H	<input type="text" value="10.0"/>	FT.	MAX WALL OPENING HT, H _c	<input type="text" value="4.5"/>	FT.		
WALL LENGTH, L	<input type="text" value="8.7"/>	FT.	QUALIFYING WALL LENGTH, L	<input type="text" value="5.7"/>	FT.	SHEARWALL ASSEMBLY	<input type="text" value="P3"/>

CAPACITY EVALUATION:

TOTAL SHEAR LOAD ON WALL	<input type="text" value="1800"/>	LBS	<	ALLOWABLE SHEARWALL CAPACITY	<input type="text" value="2552"/>	LBS
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SHEARWALL ASSEMBLY SPECIFICATION

P3 - 1-SIDE 7/16" OSB
FASTENED W/ 8D NAILS AT 3"O.C. PANEL EDGES & 12"O.C. PANEL FIELD - EDGES BLOCKED
ADEQUATE

OVERTURNING EVALUATION:

RESISTIVE DL	<input type="text" value="243"/>	PLF	OVERTURNING MOMENT	<input type="text" value="18.0"/>	K-FT	HOLD DOWN DESIGN LOAD	<input type="text" value="1406"/>	LBS
DL AT ENDS OF WALL	<input type="text" value="400"/>	LBS	RESISTIVE MOMENT	<input type="text" value="5.8"/>	K-FT	HOLD DOWN CAPACITY	<input type="text" value="3695"/>	LBS

HOLD-DOWN SPECIFICATION

SIMPSON STDH14RJ HOLDOWN

SHEARWALL 105: 1ST - REAR EXT GARAGE

SHEARWALL PROPERTIES:

WALL HEIGHT, H	<input type="text" value="8.0"/>	FT.	MAX WALL OPENING HT, H _c	<input type="text" value="0.0"/>	FT.		
WALL LENGTH, L	<input type="text" value="19.5"/>	FT.	QUALIFYING WALL LENGTH, L	<input type="text" value="19.5"/>	FT.	SHEARWALL ASSEMBLY	<input type="text" value="P1"/>

CAPACITY EVALUATION:

TOTAL SHEAR LOAD ON WALL	<input type="text" value="3400"/>	LBS	<	ALLOWABLE SHEARWALL CAPACITY	<input type="text" value="4677"/>	LBS
--------------------------	-----------------------------------	-----	---	------------------------------	-----------------------------------	-----

SHEARWALL ASSEMBLY SPECIFICATION

P1 - 1-SIDE 7/16" OSB
FASTENED W/ 8D NAILS AT 6"O.C. PANEL EDGES & 12"O.C. PANEL FIELD - EDGES BLOCKED
ADEQUATE

OVERTURNING EVALUATION:

RESISTIVE DL	<input type="text" value="287"/>	PLF	OVERTURNING MOMENT	<input type="text" value="27.2"/>	K-FT	HOLD DOWN DESIGN LOAD	<input type="text" value="0"/>	LBS
DL AT ENDS OF WALL	<input type="text" value="1000"/>	LBS	RESISTIVE MOMENT	<input type="text" value="34.1"/>	K-FT	HOLD DOWN CAPACITY	<input type="text" value="0"/>	LBS

HOLD-DOWN SPECIFICATION

NO HOLDOWN REQUIRED



SHEARWALL DESIGN SUMMARY

SHEARWALL XXX: - NOT USED

SHEARWALL PROPERTIES:

WALL HEIGHT, H FT. MAX WALL OPENING HT, H_c FT.
WALL LENGTH, L FT. QUALIFYING WALL LENGTH, L FT. SHEARWALL ASSEMBLY

CAPACITY EVALUATION:

TOTAL SHEAR LOAD ON WALL LBS ALLOWABLE SHEARWALL CAPACITY LBS
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SHEARWALL ASSEMBLY SPECIFICATION

PO - 1-SIDE 7/16" OSB
FASTENED W/ 8D NAILS AT 6"O.C. PANEL EDGES & 12"O.C. PANEL FIELD - UNBLOCKED
#DIV/0!

OVERTURNING EVALUATION:

RESISTIVE DL PLF OVERTURNING MOMENT K-FT HOLD DOWN DESIGN LOAD LBS
DL AT ENDS OF WALL LBS RESISTIVE MOMENT K-FT HOLDOWN CAPACITY LBS

HOLD-DOWN SPECIFICATION

NO HOLDOWN REQUIRED

SHEARWALL 106: 1ST - SIDE EXT GARAGE

SHEARWALL PROPERTIES:

WALL HEIGHT, H FT. MAX WALL OPENING HT, H_c FT.
WALL LENGTH, L FT. QUALIFYING WALL LENGTH, L FT. SHEARWALL ASSEMBLY

CAPACITY EVALUATION:

TOTAL SHEAR LOAD ON WALL LBS < ALLOWABLE SHEARWALL CAPACITY LBS

SHEARWALL ASSEMBLY SPECIFICATION

P1 - 1-SIDE 7/16" OSB
FASTENED W/ 8D NAILS AT 6"O.C. PANEL EDGES & 12"O.C. PANEL FIELD - EDGES BLOCKED
ADEQUATE

OVERTURNING EVALUATION:

RESISTIVE DL PLF OVERTURNING MOMENT K-FT HOLD DOWN DESIGN LOAD LBS
DL AT ENDS OF WALL LBS RESISTIVE MOMENT K-FT HOLDOWN CAPACITY LBS

HOLD-DOWN SPECIFICATION

NO HOLDOWN REQUIRED



SHEARWALL DESIGN SUMMARY

SHEARWALL 107: 1ST - SIDE EXT DINING

SHEARWALL PROPERTIES:

WALL HEIGHT, H FT. MAX WALL OPENING HT, H_c FT.
WALL LENGTH, L FT. QUALIFYING WALL LENGTH, L FT. SHEARWALL ASSEMBLY

CAPACITY EVALUATION:

TOTAL SHEAR LOAD ON WALL LBS < ALLOWABLE SHEARWALL CAPACITY LBS

SHEARWALL ASSEMBLY SPECIFICATION

P3 - 1-SIDE 7/16" OSB
FASTENED W/ 8D NAILS AT 3"O.C. PANEL EDGES & 12"O.C. PANEL FIELD - EDGES BLOCKED
ADEQUATE

OVERTURNING EVALUATION:

RESISTIVE DL PLF OVERTURNING MOMENT K-FT HOLD DOWN DESIGN LOAD LBS
DL AT ENDS OF WALL LBS RESISTIVE MOMENT K-FT HOLDOWN CAPACITY LBS

HOLD-DOWN SPECIFICATION

SIMPSON STDH14RJ HOLDOWN

SHEARWALL 108: 1ST - SIDE INT GARAGE

SHEARWALL PROPERTIES:

WALL HEIGHT, H FT. MAX WALL OPENING HT, H_c FT.
WALL LENGTH, L FT. QUALIFYING WALL LENGTH, L FT. SHEARWALL ASSEMBLY

CAPACITY EVALUATION:

TOTAL SHEAR LOAD ON WALL LBS < ALLOWABLE SHEARWALL CAPACITY LBS

SHEARWALL ASSEMBLY SPECIFICATION

P3 - 1-SIDE 7/16" OSB
FASTENED W/ 8D NAILS AT 3"O.C. PANEL EDGES & 12"O.C. PANEL FIELD - EDGES BLOCKED
ADEQUATE

OVERTURNING EVALUATION:

RESISTIVE DL PLF OVERTURNING MOMENT K-FT HOLD DOWN DESIGN LOAD LBS
DL AT ENDS OF WALL LBS RESISTIVE MOMENT K-FT HOLDOWN CAPACITY LBS

HOLD-DOWN SPECIFICATION

SIMPSON STDH14RJ HOLDOWN



SHEARWALL DESIGN SUMMARY

SHEARWALL 109: 1ST - SIDE EXT STAIRS / GREAT ROOM

SHEARWALL PROPERTIES:

WALL HEIGHT, H FT. MAX WALL OPENING HT, H_c FT.
WALL LENGTH, L FT. QUALIFYING WALL LENGTH, L FT. SHEARWALL ASSEMBLY

CAPACITY EVALUATION:

TOTAL SHEAR LOAD ON WALL LBS < ALLOWABLE SHEARWALL CAPACITY LBS

SHEARWALL ASSEMBLY SPECIFICATION

P3 - 1-SIDE 7/16" OSB
FASTENED W/ 8D NAILS AT 3"O.C. PANEL EDGES & 12"O.C. PANEL FIELD - EDGES BLOCKED
ADEQUATE

OVERTURNING EVALUATION:

RESISTIVE DL PLF OVERTURNING MOMENT K-FT HOLD DOWN DESIGN LOAD LBS
DL AT ENDS OF WALL LBS RESISTIVE MOMENT K-FT HOLDDOWN CAPACITY LBS

HOLD-DOWN SPECIFICATION

NO HOLDOWN REQUIRED

SHEARWALL 110: 1ST - SIDE EXT COVERED PATIO

SHEARWALL PROPERTIES:

WALL HEIGHT, H FT. MAX WALL OPENING HT, H_c FT.
WALL LENGTH, L FT. QUALIFYING WALL LENGTH, L FT. SHEARWALL ASSEMBLY

CAPACITY EVALUATION:

TOTAL SHEAR LOAD ON WALL LBS < ALLOWABLE SHEARWALL CAPACITY LBS

SHEARWALL ASSEMBLY SPECIFICATION

P3 - 1-SIDE 7/16" OSB
FASTENED W/ 8D NAILS AT 3"O.C. PANEL EDGES & 12"O.C. PANEL FIELD - EDGES BLOCKED
ADEQUATE

OVERTURNING EVALUATION:

RESISTIVE DL PLF OVERTURNING MOMENT K-FT HOLD DOWN DESIGN LOAD LBS
DL AT ENDS OF WALL LBS RESISTIVE MOMENT K-FT HOLDDOWN CAPACITY LBS

HOLD-DOWN SPECIFICATION

NO HOLDOWN REQUIRED